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CASE HISTORY OF AN OPEN PIT COAL MINE SLOPE FAILURE AT LUSCAR, ALBERTA

by

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A Project

Submitted To The Faculty of Graduate Studies and Research In Partial Fulfillment of the Requirements For The Degree Master of Engineering in Geotechnical Engineering

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Department of Civil Engineering

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ABSTRACT

The case history of a major open pit coal mine slope failure is described. The site is Cardinal River Coals Ltd. near Hinton, Alberta. The magnitude of the failure was approximately one million cubic yards (one million cubic metres). A substantial amount of piezometric and slope displacement data was collected before and after failure. The slope instability was detected from surface displacement measurements more than 100 days before any visual signs of failure appeared. The slope displacements were monitored on a continuous basis, permitting safe mining operations up to several hours before failure. The slope was ultimately stabilized by a rock fill toe buttress, to permit future mining in front of the failure. Analysis of slope displacements provided insight into the detailed mechanics of the failure. Stability analyses, combined with slope displacements, provided further insight into the nature of the failure.

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This report describes the case history of a major open pit mine slope failure which occurred at Cardinal River Coals Ltd. pit 51-B-2 on November 10, 1979.

This case history is of particular interest because:

- the magnitude of the failure volume was relatively large, in the order of one million cubic yards (one million cubic metres);
- piezometric and slope displacement data was collected before and after failure, providing some insight into the details of the mechanics of failure;
- the slope instability was detected from displacement measurements long before any visible signs of failure were apparent;
- the slope displacements were monitored on a continuous basis through failure, permitting safe mining operations up to several hours before failure;
- the slope was ultimately stabilized by a rock fill buttress to permit future mining in front of the failure.

1.1 Units of Measurement

Since Cardinal River Coals Ltd. use the Imperial system of measurement for base maps and sections, the primary units utilized in the figures, text and tables of this report are Imperial for consistency. Wherever practical, metric units are provided in figures and text (in brackets). Raw data in tables is presented in the original units of the measuring instrument.

2.0 REGIONAL DESCRIPTION

2.1 Location and Access

Cardinal River Coals Ltd. is located in west-central Alberta approximately 25 miles (40 kilometres) south of Hinton and 200 miles (320 kilometres) west of Edmonton, on Alberta Highway 40 (Figure A.l, Appendix A). The mine occupies the site of the former town of Luscar.

Pit 51-B-2 is located mainly in Section 27, Township 47, Range 24 West of the Fifth Meridian.

2.2 Topography and Surface Drainage

The mine is situated along the southwestern limits of the Rocky Mountain Foothills, adjacent to the Front Ranges of the Rocky Mountains (Figure A.1).

Approximately 3 miles (5 kilometres) to the southwest, Luscar Mountain rises to 8500 feet (2590 metres) elevation. The ground surface slopes regionally to the northeast, falling to approximately 5000 feet (1525 metres) elevation at the northern edge of the mine site. Within the mine site, natural ground elevations range from approximately 5000 to 6100 feet (1525 to 1860 metres) elevation.

Open pit coal mining extends down to approximately 5000 feet (1525 metres) elevation.

Surface drainage from the site flows west to Cabin Creek, east to Luscar Creek and north to Mary Gregg Creek, which all flow to the McLeod River.

2.3 Surficial Soils and Vegetation

Surficial soil cover across the site is typically thin, in the order of 10 feet (3 metres) or less except along stream valleys. Soils are predominantly colluvium and cordilleran till, which both strongly reflect the composition of underlying sedimentary bedrock. Constituent grain sizes consist predominantly of sand, with lesser amounts of clay and silt. The soils are generally very stony and well-drained.

Along stream valleys, soil thickness can exceed 10 feet (3 metres), and concentrations of coarser and finer-grained soils are formed by fluvial processes.

Prior to mining, the area was covered with trees, predominantly spruce and lodge pine.

2.4 Climate

The Hinton area has a continental subhumid climate, with long, cold winters modified by short periods of chinook conditions and short, cool summers. The mean annual temperature is approximately 34.3° F. (1.3° C.). Mean total precipitation is approximately 20 inches (510 mm), and evapotranspiration is approximately 14.5 inches (370 mm).

2.5 Land Use

Land uses include forestry, recreation and coal mining.

Mining at the Cardinal River Coals site was started in 1921 by Luscar Coal Ltd. Mining at Luscar continued by open pit and underground methods until 1956.

Mining was recommenced by Cardinal River Coals Ltd. in 1970. In 1979, Cardinal River produced 2.7 million clean short tons (2.5 million tonnes) of bituminous coal from three open pits including Pit 51-B-2. The coal was exported for consumption by the Japanese steel industry.

2.6 Regional Structural Geology

Tectonic forces from the southwest have produced major regional folding and faulting which trends northwest - southeast. The Lower Cretaceous strata of the Luscar area lie in two large folds, the Cadomin Synclinorium and the Luscar Anticlinorium, as mapped by Hill (1980) in Figure A.2. Major faults are the Nikanassin Thrust, which outcrops along the Front Ranges to the southwest and the Luscar Thrust, which outcrops between the axes of the two folds discussed above.

The Nikanassin Thrust dips steeply to the southwest at approximately 70°, with a dip direction of approximately 210°. Fault displacement is substantial, in places thrusting Upper Devonian Limestones over Cretaceous sediments. The Luscar Thrust outcrops at the south limit of Pit 51-B-2. Displacement along this fault is approximately 2400 feet (730 metres). The fault dips to the southwest at approximately 30° in the mine area, flattening to the southeast and steepening to the northwest. Associated with the Luscar Thrust are numerous small faults which displace the Jewel coal seam by several tens of feet. It has been inferred that the Luscar Thrust and associated minor faults were folded after faulting.

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Pit 51-B-2 lies within the south limb of the Luscar Anticlinorium and is defined by a flat-bottomed syncline. The apical angle of the syncline is 140°, and the apical angle of the anticlinorium is 90°. The syncline apical angle decreases to the northwest, and the syncline is distorted into a "W" shape at the western limit of Pit 51-B-2.

Folding tends to cause thickening of the coal at the axes of the folds, which makes mining more economically attractive in these zones.

2.7 Regional Stratigraphy

Hill (1980) summarized the stratigraphic column for the Luscar area as shown in Table 2.1.

Table 2.1 - Stratigraphic Column for the Luscar Area.

·		
Paleocene	Upper	Brazeau Formation Wapiabi Formation Cardium Formation Blackstone Formation
Cretaceous	Lower	Mountain Park Formation Luscar Formation Cadomin Formation Nikanassin Formation
Jurassic		Fernie Group
Triassic		Spray River Group
Paleozoic		Undivided

The mineable coal is confined to the Lower Cretaceous Luscar Formation. The stratigraphy of the Luscar Formation is depicted in Figure A.3, and is described further below.

Hill divided the Luscar Formation into A, B, C and D members. The Luscar Formation conformably overlies the Cadomin Formation and grades into the overlying Mountain Park Formation. Total thickness of the Luscar Formation is estimated to be 1400 feet (425 metres).

Member A

Member A at the base of the Luscar Formation, consists of thin, interbedded sandstones, siltstones and shales with thin coal seams. A coarsening upward sequence is prevalent, with coal or carbonaceous shale at the base and sandstone at the top. Total thickness is estimated to be approximately 413 feet (126 metres).

The sandstones are fine to medium-grained and light to dark grey, weathering to brown, orange and buff. They are predominantly calcareous although occasionally iron-rich, and frequently contain fossils.

The siltstones are typically fissile, dark grey or black weathering to green, brown and buff.

The shales are dark grey or black and frequently carbonaceous, grading into coal seams.

Member B

Member B consists mainly of shales, coarsening near the top into interbedded shales and siltstones, siltstones, and sandstones. Total thickness is approximately 312 feet (95 metres).

The basal shales are 60 feet (18 metres) thick, dark grey in colour and contain thin limestone beds.

The overlying interbedded shales and siltstones are typically dark grey, fissile and sometimes orange weathering.

The sandstones at the top are typically fine to medium-grained, light to dark grey, noncalcareous, orange weathering and sometimes cross-bedded.

Member C

Member C consists of the highly resistant, ridge-forming"Torrens" sandstone. It is 240 feet (43 metres) thick.

The member contains a central shale parting, approximately 100 feet (30 metres) below its top surface, with the overlying sandstone massive and the underlying sandstone well-bedded. The sandstone is medium-grained, light grey, buff to orange weathering and noncalcareous. Thin siltstones and thin chert pebble conglomerates are occasionally present.

Member D

Member D consists of thick sandstones and siltstones, interbedded sandstones, siltstones and shales and thick and thin coal seams. It is 597 feet (182 metres) thick.

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At the base of this member is the Jewel coal seam, which averages 40 feet (12 metres) thick. This is the main economic coal seam. It contains three thin shale partings.

Approximately 243 feet (74 metres) above the base of Member D is the Ryder coal seam. It averages 12 feet (3.5 metres) thick and is much more shaley than the Jewel seam. The Ryder has not usually been mined.

The two coal seams are separated by thick-bedded sandstones and siltstones, with thin coal or carbonaceous shale seams occurring in or above the dark grey fissile siltstones.

The sandstones are typically light grey, weathering to orange and are mostly non-calcareous. Fine, medium and coarse-grained sizes are present in both massive and well-bedded units. The sandstones toward the top of the member are green-grey, grading into the Mountain Park Formation.

2.8 Regional Hydrogeology

Groundwater flow typically occurs along jointing and bedding. The most active groundwater flow is near surface and is strongly influenced by surface topography. Most groundwater is discharged into local topographic lows, with a small amount of recharge flowing to regional flow systems to the northeast.

Piezometric response during spring breakup or sudden rainstorms can be substantial, sometimes exceeding 100 feet (30 metres) over a few days.

Hydraulic conductivity (permeability) parallel to bedding is generally greater than across bedding, despite well-developed jointing patterns.

Due to the short groundwater flow path and slightly alkaline bedrock, groundwater quality is typically high. Total dissolved solids generally do not exceed 1,000 ppm, and carbonate-bicarbonate anions predominate.

- 3.0 SITE DESCRIPTION
- 3.1 Site Configuration
- 3.1.1 Pit 51-B-2 Configuration

Pit 51-B-2 is depicted in Figure A.4 and Plate 1.

Highway 40 crosses the 51-B pit area immediately west of Pit 51-B-2. The original topography prior to mining sloped to the west, with surface drainage flowing to Cabin Creek.

The pit is oriented Northwest-Southeast parallel to the regional structural trend, and is approximately 3000 feet by 3000 feet (900 metres by 900 metres) in size. Elevation ranged from approximately 5810 feet (1771 metres) at the highest point on the east wall to approximately 5333 feet (1625 metres) at the pit floor at the time of the November 10, 1979 slope failure. Pit design floor was 5067 feet (1544 metres).

Mining progressed in 33 foot (10 metre) lifts, with safety berms every second lift.

3.1.2 North Wall Slope Failure Configuration

The November 10, 1979 slope failure occurred in the North Wall of Pit 51-B-2, as depicted in Figure A.4 and Plates 2 to 7.

The original slope design (Golder Associates, 1975) recommended a 38° overall slope, with 30 foot (9 metre) wide safety berms at 67 foot (20 metre) intervals. The actual overall slope inclination measured through Section A-A' was 36°, Figure A.5. Berm widths were generally narrower than design width, due to back-break at the crest. Berm spacing was 67 feet (20 metres), starting from the 5633 foot (1717 metre) elevation.

The slope failure was 800 feet (245 metres) in length and contained an estimated volume of approximately one million cubic yards (1 million cubic metres) of rock. The height of the failure ranged from 290 feet (88 metres) at the west limit of the failure to 340 feet (104 metres) at the east limit. Elevation at the crest of the slope failure ranged from 5550 feet (1692 metres) at the west limit to 5670 feet (1728 metres) at the east limit. The natural slope above the failure rose gradually to approximately 5940 feet (1811 metres) elevation. The toe of the failure ranged in elevation from approximately 5367 feet (1625 metres) across the west half to approximately 5333 feet (1625 metres) across the east half.

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3.2 Site Instrumentation

Measurements of slope displacements (movements) and piezometric elevations were performed before, during and after the failure. All instrument locations are shown in Figure A.4.

3.2.1 Slope Displacement Measurements

Slope displacement measurements employed the standard Radial Survey Method using a theodolite and Electronic Distance Measurement (EDM) instrument. A one-second theodolite was used to measure horizontal and vertical angles. The EDM unit measured the "slope distance" from the instrument to each retroreflector prism target on the slope, with a rated accuracy of \pm 0.02 feet (0.005 metres). The three-dimensional coordinate of each target was calculated, and vectors of displacement derived from changes in target locations with subsequent measurements. A one-dimensional "slope distance" component of displacement was also used, since it provided somewhat higher accuracy.

The theodolite and EDM unit were mounted on a permanent concrete monument at Station KR-7 (Figure A.4). Target prisms were permanently mounted on steel posts, which were cemented into holes in the slope.

Inaccuracy in theodolite readings is generally due to pointing error.

Inaccuracy in EDM readings is generally due to inherent difficulties in measuring the atmospheric correction parameters (barometric pressure and temperature) along the line of measurement. Additional sources of error for both instruments are movement of survey reference stations, wind shake of instruments, and atmospheric heat waves.

Slope displacement data is included in Appendix B, and discussed in Section 4.0 of this report.

3.2.2 Piezometers

Standpipe piezometers were installed in individual boreholes, with two or three adjacent boreholes drilled to different depths constituting a "piezometer nest". Piezometer nests BO4 (A and B), BO7 (A,B and C) and BO8 (A, B and C) were located in the north wall as depicted in Figure A.4. In each case, instrument A was established in the shallow borehole, with B and C progressively deeper. Each piezometer consisted of a 2 inch (50mm) nominal PVC plastic standpipe, slotted for an appropriate interval near the bottom of the borehole. The slotted "screen" inlet section was surrounded by clean, coarse sand and hydraulically isolated from overlying strata by bentonite balls and cement grout to surface. Measurements were taken using an electronic "dropline" to detect the water elevation in the standpipe. Inaccuracy in a standpipe piezometer is generally due to hydraulic "plugging" of the screen inlet section, or to the misapplication of standpipes in relatively impervious ground such that the response of the instrument to changing groundwater pressure is excessively slow.

Piezometer data is included in Appendix C, and discussed in Section 4.0 of this report. The screen elevations of piezometer nests B07 and B08 are depicted in Figure A.5.

3.3 Site Structural Geology

3.3.1 Pit 51-B-2 Structural Geology

Strata in Pit 51-B-2 are folded into a broad syncline which follows the regional northwest-southeast trend and plunges gently to the southeast at an average angle of 5°.

In the north side of the pit, the coal seam is displaced downward and thickened due to an east-west striking thrust fault which dips steeply to the south, as depicted in Figure A.6. This fault is associated with tight chevron faulting and folding observed in the north end of the east pit wall. Fault displacement ranges from near zero at the east wall to approximately 100 feet (30 metres) at Section F0 + 00 in the failure area. The fault strikes subparallel to the synclinal fold structure which defines the geometry of Pit 51-B-2, such that its proximity to the north wall increases as it progresses to the northwest.

3.3.2 North Wall Structural Geology

Hebil (1980a) summarized the north wall structural geology based on 260 measurements of discontinuities made by mine and consultant's personnel in 1979. The results are depicted in plan in Figure A.7 and in a pole plot in Figure A.8, and the most significant discontinuity sets are summarized in Table 3.1.

Table 3.1 - North Wall Structural Discontinuities

Discontinuity	<u>Strike</u>	Dip	Dip Direction
Bedding	114°	38°S	204°
Joint Set J _l	213°	79°NW	303°
Joint Set J ₂	281°	54°N	11°
Joint Set J ₃	164°	70°E	74°
Joint Set J4	143°	58°E	53°

Hebil divided the north wall into east and west portions at approximately 104,600E, the eastern limit of the slope failure. The average bedding dip of 38° was generally uniform, ranging between 35 to 40°. Major variations in bedding have been caused by structural features of local extent.

The east portion of the north wall was generally more uniform and planar than the west portion, averaging 38° bedding dip. This is believed to be due to the greater distance between the wall and the east-west trending thrust fault discussed above. However, folding and faulting associated with this thrust fault may intersect the east portion of the wall at greater depth. A northeast trending structural zone intersected the east end of the north wall causing local abrupt changes in bedding orientation, but was too far to the east to contribute to the failure.

The west portion of the wall included the slope failure. Bedding dip in the upper half of the west portion was relatively uniform, averaging 38°. The lower half of the west portion exhibited much more variable bedding. There were widespread observations of bedding dipping out of the slope at 25°, which undercut the overall slope, and steeply dipping upright and overturned beds at the toe of the slope. It was hypothesized that these features were associated with the thrust fault discussed above. It is believed that these features observed in the lower half of the slope were directly involved in the failure, as discussed further below.

Joint sets J1 and J2 were by far the most prominent in terms of frequency of occurrence. Sets J1 and J2 are essentially orthogonal with bedding, and are aligned with the northwest-southeast trending regional structural trend. The steep northwesterly dip of J1 correlates with the 5° average southeasterly plunge of the syncline.

The main fault mechanism observed in this area is thrusting along bedding planes. Moderately to intensely sheared surfaces exist on set J1, and to a lesser extent on J3 and J4. Set J1 joint spacing averaged approximately 1 to 3 feet (0.3 to 1 metre) in the north wall, but J1 frequency increased by approximately tenfold at the east limit of the failure.

Set J2 is generally not sheared. However, where bedding is steeply overturned at the toe of the failure, J2 could act as a plane of weakness dipping out of the slope at a shallow angle, thus facilitating failure surface development (Figure A.6).

3.4 North Wall Stratigraphy and Lithology

Pit 51-B-2 strata are situated entirely within the Luscar Formation, and the Jewel coal seam is the only seam of economic significance.

Information on lithology in the north wall is scant due to limited exposure and coreholes. Surficial soil typically consisted of sandy colluvium, in the order of 10 feet (3 metres) thick. A single corehole, number E21-R50 was drilled by Golder Associates in 1975 for design of the north wall. This hole is 900 feet (274 metres) east of Section F0 + 00, as shown in Figure A.4, and has been extrapolated onto section in Figure A.6. The corehole log is included in Appendix D.

Bedrock exposures at the surface of the north wall were predominantly sandstone, locally interbedded with thin siltstone and coal seams. Bedding plane partings were slickensided. An east trending linear depression in the crest of the north wall original ground surface coincided with the location of the tension crack. It was hypothesized by Hebil (1980a) that this depression coincided with weaker, more erodable beds deeper in the wall, which could have contributed to slope instability.

There is some support for this hypothesis from corehole E21-R50. At a depth of 133 to 141 feet (40.5 to 43.0 metres), which coincides with the inferred location of the failure surface, a zone of interbedded soft carbonaceous clay was observed in the core. Clay infilling of fractures was observed repeatedly along the length of the corehole, and occasional coal stringers were also present. RQD (rock quality designation) from 122 to 142 feet (37.2 to 43.3 metres) ranged as low as 20%. Since the overall geologic structure plunged approximately 5° to the southeast, this weak zone would be expected to occur at a somewhat higher elevation in the failure area.

In summary, lithology in the footwall consisted primarily of sandstones and siltstones, with frequent clay infilling of bedding fractures, occasional coal stringers, and a soft carbonaceous clay layer in the vicinity of the failure surface.

3.5 Site Hydrogeology

Piezometers in the slopes of open pit mines at Cardinal River Coals Ltd. typically exhibit downward hydraulic gradients. The shallowest (A) piezometers respond rapidly to surface runoff and precipitation, and the deeper piezometers respond more slowly or not at all. During spring thaw, increases in groundwater elevations in shallow piezometers of up to 100 feet (30 metres) are not uncommon.

Piezometer nests BO4 (A and B), BO7 (A, B, and C) and BO8 (A, B, and C) are depicted in Figures A.4 and A.5. Piezometric data is summarized in Appendix C.

In Figures C.1 to C.3, piezometric elevations are plotted with respect to time. Piezometric elevations in early March 1979 (Day 60) ranged from 5587 down to 5489 feet (1702.9 to 1673.0 metres). This is the seasonal low for groundwater elevations, and the time when flow systems are closest to steady-state conditions.

Certain anomalous behaviour was apparent in the north wall piezometers. In early March (Day 60), piezometers B07B and B08B exhibited strongly artesian conditions with groundwater elevations approximately 60 feet (18 metres) higher than the adjacent A and C piezometers. The similarity in profile of B04B and B07B, and the slight hydraulic gradient to the west from B04B to B07B to B08B, indicate that these piezometers intersected the same artesian
aquifer along strike, and that recharge was primarily from the east. This inference correlates with the local topographic high up bedding dip to the northeast. It is also apparent that piezometer BO7A is plugged, which was confirmed by response tests.

The substantial hydraulic gradient across bedding indicates that hydraulic conductivity (permeability) across bedding is much less than that parallel to bedding.

The rapid response of piezometers B07A and B08A to spring thaw is apparent starting in early April (Day 94), when levels in both piezometers rose approximately 50 feet (15 metres). A smaller, delayed response is observed in deeper piezometers B04B and B07B.

Further comments on groundwater conditions related to the slope failure will be included in Section 4.0 of this report.

4.0 ANALYSIS OF SLOPE FAILURE

The following section includes an overview summary of events, a more detailed analysis of the mechanics of the failure, and a computer back-analysis of the failure.

Due to the very large amount of data available from Pit 51-B-2, only that data considered directly pertinent to the failure is included in this report. Certain slope displacement targets on the north wall were deleted (for example, targets which were lost in the early stages of movement, or targets which were distant from the failure). Unfortunately, much of the original "slope distance" data obtained by contract surveyors from the continuous monitoring of the slope has been lost; all remaining data has been included. Dewatering data was sparse and of little practical value to this analysis; it has not been included, although the results are apparent in the piezometric data.

The rate of progress of mining is apparent in Figures A.10 to A.20 inclusive, Appendix A.

Slope displacement data is summarized in Appendix B and piezometric data is included in Appendix C.

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4.1 Summary of Major Events

A brief overview of the major events of this case history is presented, as these events were interpreted in monthly site inspection reports by the project geotechnical consultants. A more detailed analysis of events with the benefit of hindsight is provided in Section 4.2.

Time is represented in days, starting with Day 1 on January 1, 1979.

Prior to the beginning of 1979, the north wall of Pit 51-B-2 exhibited no significant movement, with the exception of a small localized slope failure in the area of Target 22B. The adverse orientation of bedding with respect to slope stability had been identified. The artesian piezometric condition in the north wall had also been identified, and recommendations for slope dewatering were submitted.

From January 28 (Day 28) to May 8, 1979 (Day 128), a slope displacement of 0.05 feet (0.015 metres) was observed in Target 26B. This apparent slope movement was too close to measurement system accuracy for conclusive analysis. Piezometers B07A and B08A reached annual minimum elevations of approximately 5500 feet (1676 metres) on or about March 1 (Day 60), and then rose sharply due to spring thaw.

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From May 8 (Day 128) to June 11 (Day 162), movement occurred at all north wall targets, with the maximum observed change at Target 26B equal to 0.20 feet (0.061 metres) over this time interval.

Piezometers B07A and B08A reached their annual maximum elevations of approximately 5550 feet (1692 metres) in this period, approximately 50 feet (15 metres) above annual minimum elevations. Mining immediately below the north wall was considered a major contributing factor to these slope movements. The north slope was flagged as an area requiring particular vigilance.

From June 11 (Day 162) to July 9 (Day 190), movements at all targets on the north wall progressed more slowly, with the maximum change of 0.05 feet (0.015 metres) occurring at Target 26B. Cumulative movement in Target 26B was now 0.31 feet (0.094 metres) in the horizontal component, and continued movement of the north wall was identified as cause for major concern as the pit deepened.

From July 9 (Day 190) to August 13 (Day 225), slope movements were relatively insignificant. The north wall was reported to be temporarily stable, but renewed movement was predicted.

From August 13 (Day 225) to September 4 (Day 247), major renewed movement was observed, with the maximum change of 0.29 feet (0.088 metres) at Target 26B, and lesser changes at Targets 21B and 22B. Cumulative displacement at Target 26B was now 0.60 feet (0.183 metres) in horizontal component at an azimuth of 245° (trending downslope to the west), and 0.21 feet (0.064 metres) downward in vertical component. Between August 14 (Day 226) and September 6 (Day 249), Piezometer B08B was observed to fall sharply from 5547 to 5507 feet (1691 to 1679 metres). On September 11 (Day 254), the north slope area was inspected in detail. There was no visual evidence of any slope movement. Concern with regard to measured slope movements was restated, and recommendations were made to perform a more detailed evaluation of north wall slope stability and to install additional targets for slope displacement measurements.

From September 4 (Day 247) to October 4 (Day 277), movements at all targets on the north wall progressed more slowly. The maximum change of 0.15 feet (0.046 metres) in horizontal component occurred at Target 26B.

From October 4 (Day 277) to October 22 (Day 295), movement at Target 26B accelerated, changing 0.23 feet (0.070 metres) in horizontal component. On October 26 (Day 299), a 33 foot (10 metre) lift was blasted immediately below Target 26B.

Between October 22 (Day 295) to October 29 (Day 302), movement at Target 26B accelerated sharply, changing 0.70 feet (0.213 metres) in horizontal component. A large tension crack was observed at the crest of the slope, confirming that a full height slope failure was in progress.

The following steps were initiated:

- access of men and equipment in front of the failure was stopped, and a rock window barricade placed across the pit floor;
- surface water infiltration into the tension crack was controlled by surface grading;
- remedial dewatering was commenced to attempt to stabilize the failure;
- additional displacement measurement targets were installed;
- continuous survey displacement monitoring was established;
- a "critical slope velocity" of 0.10 feet/hour (0.030 metres/ hour), measured in "slope distance" component, was established, at which point all pit personnel and equipment would be evacuated;

Of the targets existing prior to October 31 (Day 304), only 26B was located on the failure. On October 31, Targets 37B to 40B were installed on the failure.

From October 31 (Day 304) to November 10 (Day 314) at 9:00 hours, movements of all targets on the failure strongly accelerated. Target 26B on the east end of the failure moved the least, changing 2.68 feet (0.817 metres) in horizontal component. Movement increased toward the west end of failure, changing a maximum of 4.94 feet (1.505 metres) in horizontal component at Target 37B.

On November 10 (Day 314) at approximately 8:00 hours, the critical slope velocity was attained. Pit personnel and equipment were evacuated. Failure cumulative displacement to this time was approximately 7 feet (2 metres) maximum horizontal component at Target 37B.

At approximately 13:00 hours, rapid slope failure occurred. Maximum displacement of an additional 16 feet (4.9 metres) horizontal component occurred at Target 37B, after which the failure decelerated.

On November 11 (Day 315), operations resumed in the east half of the pit, distant from the failure, under continuous survey displacement monitoring. A new "critical slope velocity" of 0.05 feet/hour (0.015 metres/hour) was established.

During November and December, attempts were made to dewater the slope to enhance stability. Due to access and cold weather difficulties, dewatering progress and effectiveness was limited. A rock fill buttress was proposed to stabilize the slide, with continued dewatering as a secondary stabilization measure.

In January 1980, the rock buttress was placed in front of the slide, with the aid of continuous survey displacement monitoring. By January 31 (Day 396) slope movement had ceased.

During the time of this slope failure, mining of Pit 51-B-2 continued safely with a loss of only 1 day of production on November 10. Mining was completed with a minimal redesign and no loss of coal.

4.2 Detailed Analysis of Events

The following detailed analysis of events has been developed with the benefit of hindsight, and utilizes information summarized in Appendices A to C inclusive. In order to obtain the highest possible accuracy, one-dimensional "slope distance" measurements are used where available instead of three-dimensional components, which exhibit slightly lower accuracy. Mining progress is expressed in terms of 33 foot (10 metre) "lifts", with fractional lifts being used to indicate the approach of a subsequent lift to the north wall.

From March 31, 1978 to March 6, 1979 (Day 65), Targets 21B and 23B exhibited no measurable movement. Target 22B moved approximately 2.1 feet (0.6 metres) in a small, single bench scale failure. The failure was approximately 150 feet (45 metres) long and the tension crack was a maximum of 30 feet (9 metres) from the crest of the 5633 foot (1717 metre) bench. The failure was due to adverse localized bedding orientation and temporary surface water ingress, and was not a contributing factor to the November 10, 1979 failure.

From March 6 (Day 65) to May 8 (Day 128), Targets 21B to 26B all moved 0.05 feet (0.015 metres) or less, as depicted in Figure B.13c. This movement was within the range of accuracy of the measurement system and no true slope movement is inferred. During this time Piezometer B08A rose by 48 feet (14.6 metres) to its seasonal maximum of 5553 feet (1693 metres), as depicted in Figure C.3. Mining consisted of 1 1/2 lifts across the west half of the November 10 future failure area, and 1/2 lift across the rest of the north slope.

It is apparent that, to this time, neither high seasonal groundwater conditions nor major mining below the slope had any significant impact on stability.

From May 8 (Day 128) to May 29 (Day 149), Targets 21B to 26B all moved rapidly. Cumulative displacements correlated with distance to the west along the slope, ranging from 0.063 feet (0.019 metres) at Target 25B to 0.178 feet (0.054 metres) at Target 26B. Piezometer B08A fell by 18 feet (5.5 metres) to 5535 feet (1687 metres) while B07A remained high. Mining consisted of 1 1/2 lifts across the west half of the failure to elevation 5433 feet (1656 metres) and 1/2 lift across the rest of the north slope to 5400 feet (1646 metres).

It is apparent that mining below 5500 feet (1676 metres) in the west half of the failure and below 5433 feet (1659 metres) to the east, was the primary change which affected slope stability during this time. However, the relative importance of groundwater as a destabilizing factor is not being disregarded, and will be quantified further in the Stability Analysis, Section 4.4.

The lack of symmetry between piezometers B08A and B07A is of interest, since these instruments displayed virtually identical

behavior in 1978 (1978 data not shown). It is possible that near-surface ground displacements in the western portion of the slope enhanced drainage in this area.

From May 29 (Day 149) to July 30 (Day 211), virtually no movement occurred in any target. Piezometer B08A continued to fall by 25 feet (7.6 metres) to 5510 feet (1679 metres). Mining was relatively inactive, removing less than 1/2 lift across the slope.

The correlation of slope displacement with mining activity remains strong.

From July 30 (Day 211) to August 27 (Day 239), Targets 21B to 26B all moved rapidly. Cumulative displacements again correlated with distance to the west along the slope, ranging from 0.150 feet (0.046 metres) at Target 23B to 0.351 feet (0.107 metres) at Target 26B. Piezometer B08A continued to decline by 6 feet (1.8 metres) to 5504 feet (1678 metres). Piezometer B08B fell sharply from its artesian level of 5547 feet (1691 metres) on August 14 (Day 226) to 5511 feet (1680 metres) by August 27 (Day 239). Mining in this period excavated 1 lift below the future failure to elevation 5400 feet (1646 metres), and approximately 1/2 lift to the east to elevation 5367 feet (1636 metres).

The correlation of slope displacement with mining activity continues to gain strength. The magnitude of displacements was increasing for equivalent mining increments, which could indicate that the pit floor was approaching a structural discontinuity which strongly contributed to the displacements. The entire slope continued to move essentially as a single block - the failure volume which included Target 26B had not yet broken away from the remaining slope, although movements continued to be greatest at the west side of the slope.

The cause of the rapid fall of Piezometer B08B is of interest, and is discussed further below. For the purpose of slope stability analysis, it is reasonable to assume a groundwater elevation of approximately 5500 feet (1676 metres) at the B08 piezometer locations from this time onward.

From August 27 (Day 239) to September 10 (Day 253), Target 26B accelerated to 0.519 feet (0.158 metres), while the remaining targets were comparatively inactive (Figures B.13b and B.13c). This could indicate the initial separation of the failure volume from the rest of the slope along the subsequently observed surface parallel to joint set J1.

From September 10 (Day 253) to October 4 (Day 277), movement of all targets continued at a somewhat slower rate than during August. Cumulative displacements ranged from 0.163 feet (0.050 metres) at Target 23B, increasing to the west to 0.624 feet (0.190 metres) at Target 26B. Piezometer B08B fell to 5501 feet (1677 metres). Mining excavated 1 lift below the east half of the failure, to elevation 5367 feet (1636 metres) and 1/2 lift further to the east.

The conclusions drawn from August data continue to be valid.

From October 4 (Day 277) to October 22 (Day 295), movement of Target 22B accelerated to 0.629 feet (0.192 metres) and Target 26B accelerated to 0.824 feet (0.251 metres), while the remaining targets were inactive. Piezometric elevations were unchanged. Mining had excavated 1 lift from the west half of the failure to 5367 feet (1636 metres) elevation, and had completed the 1/2 lift to the east to 5333 feet (1625 metres) elevation.

It is clear that the failure volume had separated from the remainder of the slope, and Target 26B was accelerating.

The small bench scale failure of 1978, on which Target 22B was located, was immediately adjacent to the new larger failure and was being dragged along due to the "edge effect" of the new failure.

To this point, no surface cracking was apparent from visual inspection.

Mining continues to correlate strongly with slope displacement.

On October 26 (Day 299), the lift from elevation 5367 feet (1636 metres) to 5333 feet (1625 metres) was blasted across the east half of the failure.

By October 29 (Day 302), Target 26B displacement had accelerated to 1.457 feet (0.444 metres), and average velocity was 0.004 feet/hour (0.001 metres/hour). The tension crack delineating the failure became visible. As detailed in Section 4.1, safety and remedial measures were implemented. A "critical slope velocity" of 0.10 feet per hour (0.030 metres/ hour) was established. Actual slope velocities at individual targets are depicted in Table B.5 and plotted in Figures B.15 to B.19. The informational basis for selection of the critical slope velocity was quite limited. The fact that no further work was conducted in front of the failure permitted the use of a relatively high critical velocity. Also, since the failure encompassed the full slope height and was "dozing" the toe in front, it was unlikely that the failure would move excessively far or fast. The selected "critical slope velocity" allowed several hours to evacuate the pit, which was highly desirable from the standpoint of labour and government relations as well as for safety reasons.

On November 10 (Day 314), rapid slope failure occurred following a steady acceleration of target displacements. As measured previously, displacement was greatest at the west end of the failure. As of 13:45 hours on November 10, the cumulative "slope distance" displacements (Figure B.14a) on the failure, proceeding from east to west were:

Target 26B - 12.008 feet (3.660 metres) since March 6, 1979, Target 38B - 12.900 feet (3.932 metres) since Oct. 31, 1979, Target 40B - 14.888 feet (4.538 metres) since Oct. 31, 1979, Target 37B - 19.803 feet (6.036 metres) since Oct. 31, 1979, Target 39B - 17.497 feet (5.333 metres) since Oct. 31, 1979. The behaviour of Target 39B was slightly anomalous. This is

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discussed further in Section 4.3, together with a more detailed analysis of three-dimensional slope displacements.

From November 10 (Day 314) to January 16, 1980 (Day 381), the slope decelerated steadily, as depicted in Figure B.5b. The timing of buttress placement is estimated to be from approximately January 2 (Day 367) to January 20 (Day 385), as inferred from the last date of reading of targets as follows:

> Target 39B - January 7 (Day 372), Target 37B - January 10 (Day 375), Target 40B - January 10 (Day 375), Target 38B - January 16 (Day 381).

It is apparent that the failure was decelerating significantly before the buttress was placed. There is no doubt, however, that the buttress arrested the movement and provided an additional degree of safety for future mining operations.

The impact of dewatering on slope stability was minor. Practical problems associated with access and winter conditions made dewatering of the upper strata in contact with Piezometer BO8A relatively ineffective. The 120 foot (37 metre) fall of BO8C in response to pumping may indicate that dewatering of upper horizons could have been effected under less adverse conditions. The high contrast in hydraulic conductivity across bedding versus parallel to bedding is demonstrated by this data (Figure C.3). The rapid fall of Piezometer B08B in August 1979 is due to one of two possible, if improbable, causes: either slope displacements directly affected the strata near the inlet screen, which is 150 feet (45 metres) stratigraphically below the inferred failure surface; or the piezometer standpipe sheared near the ground surface in response to movement, while the adjacent Piezometer B08C remained intact.

This analysis, derived with the benefit of hindsight, is not materially different from that derived from monthly consultants reports in Section 4.1. This fact strongly supports the continued practise of slope displacement monitoring as a key predictive and analytical tool for open pit mine slope stability management.

4.3 Detailed Analysis of Slope Displacements

Target displacements are tabulated and plotted in Appendix B.

Displacement directions (azimuths and inclinations) were remarkably constant over the full range of displacement. Initially, variability was greatest due to the small size of the cumulative displacement, which was of the same order of magnitude as measurement system accuracy.

The true magnitudes of target displacements and the displacement directions are summarized in Table 4.1, and plotted in Figure A.19.

The azimuths for Targets 21B to 26B, which were oriented to the west of the downslope direction, appear valid and must reflect some stress relief in that direction as a result of mining. The additional westerly movement of Target 26B, as compared with Targets 21B to 25B probably reflects a degree of dilation occurring along the near-vertical eastern failure surface parallel to joint set Jl. Azimuths of targets on the failure can be "grouped" by elevation, which logically correlates with individual beds which can slide over one another - Target 37B and 38B azimuths are 208° and 206°, respectively, and Target 39B and 40B azimuths are 202° and 200°, respectively.

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Table 4.1 - Magnitude and Direction of Target Displacements

	Time Interval	Target Number	Magnitude feet (m)	Azimuth degrees*	Inclination degrees*
Mar. to Jan.	6, 1979 (Day 65) 3, 1980 (Day 368)	21B	0.669 (0.204)	250	20
	н	22B	1.148 (0.350)	240	30
	#1	23B	0.343 (0.105)	250	20
	H	25B	0.351 (0.107)	260	15
	6, 1979 (Day 65) 31, 1979 (Day 304)	26B	2.030 (0.619)	239	23.5
	31, 1979 (Day 304) 7, 1979 (Day 372)	26B	23.941 (7.297)	223	30.7
	16	37B	40.550 (12.360)	208	24.9
	11	38B	27.409 (8.354)	206	23.7
	11	39B	36.099 (11.003)	202	31.4
	н	40B	29.632 (9.032)	200	14.5
Nov. to Nov.	3, 1979 (Day 307) 9, 1979 (Day 313)	42B	1.232 (0.376)	198	-10.7° (upward)

* Azimuths and inclinations for Targets 21B to 25B have been rounded to the nearest 5° to reflect actual accuracy at small displacements.

The inclinations of Targets 26B, 38B, 40B and 42B were respectively 30.7°, 23.7°. 14.7°, and - 10.7° (upward). These targets form a nearly vertical section through the slope. The gradually decreasing inclination of these targets, the magnitude and the consistency of inclinations with time tend to support the hypothesis that these inclinations were parallel to the curved failure surface. An inferred failure surface parallel to these movements is depicted in Figure A.5.

The inclination of Target 37B was 24.9°, which correlates well with Target 38B, at the same elevation to the east. The inclination of Target 39B was 31.4°, which does not correlate with Target 40B to the east. The magnitude of Target 39B displacement was also anomalous, as discussed further below.

The magnitudes of displacements at Targets 21B to 25B adjacent to the failure logically correlate with proximity to the failure. As previously stated, Target 22B was situated on a bench-scale failure which moved alongside the November 10 failure, and so exhibited disproportionally large displacements.

As stated previously, displacement was greatest in the western portion of the failure, with the maximum total vector of 40.6 feet (12.36 metres) observed at Target 37B. Target 38B, at the same elevation and 334 feet (102 metres) to the east, moved only 27.4 feet (8.35 metres). Total displacements through the vertical section at Target 38B were "normalized" to remove the effect of the observed increase in displacement toward the west. The resulting normalized total displacement vectors of Targets 26B, 38B and 40B are 26.8 feet, 27.4 feet, and 26.7 feet (8.17 metres, 8.35 metres, and 8.14 metres, respectively). Within the accuracy limitations of the normalizing technique, these numbers are identical.

The magnitude of Target 39B displacement was 4.5 feet (1.37 metres) less than that of nearby Target 37B. As stated previously, the inclination of Target 39B displacement was also anomalous. It is apparent that Target 39B was involved in a bench scale failure which separated from the total failure. Since total displacement of Target 39B is less than Target 37B, the main failure surface at the west end of the failure must lie above Target 39B. The absence of movement at Target 43B supports but does not conclusively prove this hypothesis.

The use of velocities calculated from displacements to assess impending slope failure involves certain complications, which must be recognized. The reading interval between displacements must be sufficiently large to permit displacements well in excess of measurement accuracy, or the resulting velocities will be misleading. This phenomenon is responsible for some of the "scatter" in velocities, as apparent in Figures B.15 to B.19. The use of the same time interval between subsequent readings helps to control this potential problem. This was practised in the field, but unfortunately not all the hourly readings were available for this report. Accordingly, velocity data presented herein must be evaluated with allowance for the above limitation.

The displacements in Targets 21B, 23B and 25B after the November 10 failure, as depicted in Figure B.13c, exhibit "scatter" in displacements ranging in excess of 0.10 foot (0.003 metres), substantially larger than demonstrated measurement system accuracy. Although not conclusively proved, it is concluded that the above displacements were real, and reflect "chatter" of the stable slope in response to "slip-stick" action of the adjacent failure along the near-vertical eastern failure surface parallel to joint set J1.

4.4 Stability Analysis

A back-analysis of the slope failure was performed with the GEOSLOPE computer program, which utilizes the Janbu analytical procedure. The analysis was run through an idealized section A-A' as depicted in Figure A.9. Detailed data on individual stability analyses is included in Appendix E.

The analysis was run for several different failure surfaces, piezometric surfaces, and material strength properties, for slope geometries without and with the stabilizing rock fill buttress. These trials are summarized in Table E.1.

The initial failure surface analysed was a bilinear surface, with the lower surface assumed to be in disturbed bedrock dipping at a shallow angle out of the slope and the upper surface parallel to the average bedding angle. The disturbed bedrock surface was assumed to have a friction angle of 25°, and the bedding effective friction angle was back-calculated to 35°, at a factor of safety of 0.985. The stablizing impact of the buttress was substantial, raising the factor of safety to 1.433. In comparison, dewatering to lower the piezometric surface by 40 feet (12.2 metres) produced a factor of safety of only 1.145. Lowering the piezometric surface by 80 feet (24.4 metres) produced a factor of safety of 1.259. A buttress half the height of the actual buttress produced a safety factor of 1.269. The second failure surface investigated was a curved surface parallel to the measured displacements of the slope (labelled Curve l in Table E.l). This surface exhibited a factor of safety of 1.121, 14% higher than the equivalent analysis which utilized the bilinear surface. This difference is, however, due largely to the longer length of the bilinear surface in the 25° disturbed There is no significant higher degree of stability of material. the curved surface over the bilinear surface. The stabilizing impact of the buttress is of similar magnitude for both surfaces. The curved surface was back-analysed at a factor of safety of 0.986, to yield a friction angle approximately parallel to bedding of 31.3°.

Other failure surfaces investigated included a modified curved surface (Curve 2) with no upward component at the toe, and a series of circular surfaces. Both displayed factors of safety slightly higher than Curve 1.

The above analyses demonstrate the relative insensitivity of the factor of safety to minor geometric variations in the failure surface, and its higher sensitivity to frictional properties along the failure surface. Additional runs were performed using a single shear strength for upper and lower portions of the rock slope, in both drained and undrained modes. The bilinear surface demonstrated a back-calculated friction angle of 32.1°, and a factor of safety after buttress completion of 1.550. For the curved failure surface, the back-calculated friction angle was 30.8°, and the factor of safety after buttress completion was 1.449.

Undrained ($\oint = 0$) analyses were run since there were clay seams in the slope near the failure surface. For the bilinear surface, an undrained shear strength of 3900 pounds per square foot (187 kPa) was back-calculated. While this figure is not considered realistic, the impact of using the undrained analysis on the factor of safety after buttress placement is significant. With the buttress, the factor of safety for the undrained case is only 1.053, versus 1.550 for the corresponding drained case. If only part of the failure surface behaved for some time in the undrained mode, the stabilizing effect of the buttress would have been decreased and delayed as excess pore pressures dissipated.

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5.0 CONCLUSIONS

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- 1. Using displacement monitoring methods which measure the movements on the surface of the slope, the following practical accomplishments were achieved:
 - The November 10, 1979 (Day 314) failure was flagged as an area requiring particular vigilance as early as June 11 (Day 162), 152 days before failure.
 - ii) By July 9, (Day 190) continued movement of the north wall was confirmed as a cause of major concern as the pit deepened. This was 112 days before the appearance of any visual signs of failure, and 124 days before failure occurred.
 - iii) Pit 51-B-2 operations were safely conducted up to several hours before failure, and only one day of production from the pit was lost due to the failure.
 - iv) The stabilizing buttress was safely placed in front of the moving failure, with no danger to men or equipment.
 - Analysis of slope displacements provided insight into the detailed mechanics of the failure:
 - Prior to the separation of the failure volume from the rest of the slope, the entire slope displaced horizontally. Movement to August 13 (Day 225), was 0.159 feet (0.048 metres) at Target 23B, increasing to the west to 0.311 feet (0.095 metres) at Target 26B.

- ii) Initial separation of the failure volume from the rest of the slope occurred between August 13 (Day 225) and September 4 (Day 247).
- iii) Following separation of the failure volume, the stable slope to the east displaced horizontally inward and outward, reflecting "chatter" in response to "slip-stick" actions of the adjacent moving failure.
- iv) Slope displacements correlated strongly with mining activity. This could indicate that the pit floor was approaching a structural discontinuity which strongly contributed to the displacements, although other valid interpretations are possible.
- v) Placement of the buttress decelerated the failure gradually to a stop. The buttress was more than half placed by January 16 (Day 381), and was fully placed by January 21 (Day 386), but movements continued until at least January 31 (Day 396).
- vi) The failure volume moved as a single intact volume, with the exception of the area near Target 39B which was part of a separate smaller failure. Total displacement of the failure volume ranged from approximately 26 feet (7.9 metres) at Target 26B, increasing to the west to approximately 45 feet (13.7 metres) at Target 37B.
- vii) It is believed that the consistent inclinations displayed by Targets 26B, 37B, 38B and 40B are parallel to the actual failure surface.

- 3. The results of the stability analyses, combined with the slope displacement data, provide further insight into the mechanics of the failure:
 - As noted above, the failure decelerated slowly in response to buttress placement. In the drained analysis, the high factor of safety demonstrated after buttress placement would lead to a rapid cessation of failure movement. It is concluded that at least part of the failure surface material acted in the undrained mode.
 - ii) While there are many variables involved which lead to a wide range of possible results, the back-calculated average effective angle of friction along the failure surface is believed to range from 30° to 32°.
 - iii) The factor of safety after buttress placement will ultimately equal 1.4 to 1.5, after dissipation of excess pore pressures along the failure surface. The failure could, however, be susceptible to renewed movements due to excess pore pressures generated by blast vibrations.
- 4. Piezometric data yielded the following conclusions:
 - Displacement of the slope prior to failure resulted in dewatering of the strata containing Piezometer BO8A in May 1979. This is inferred to be due to the opening of jointing in response to slope deformation.
 - ii) The cause of the fall in Piezometer B08B in August 1979 is less obvious. This is possibly due to slope deformation causing dewatering of the strata, but the depth is too great to strongly support this hypothesis.

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PHOTOGRAPHIC PLATES



PLATE 1 - Pit 51-B-2 North and East Walls - Looking Northeast Early Nov. 1979 - Before Failure





PLATE 2

North Wall Slope Failure - Looking North Approx. Nov. 12,1979 - After Failure



PLATE 3

North Wall Slope Failure - Looking Northwest Approx. Nov. 12,1979 - After Failure NOTE: Observe Windrows on Pit Floor to Prevent Access Below Failure, Plate 3



PLATE 4

North Wall Slope Failure - Looking East Approx. Nov. 12,1979 - After Failure



PLATE 5

North Wall Slope Failure - Close-Up of West Limit Looking East Approx. Nov. 12,1979 - After Failure

NOTE: For Scale, Observe the Power Pole, Foreground of Plates 4 & 5



PLATE 6

North Wall Slope Failure - Looking West Approx. Nov. 12,1979 - After Failure



PLATE 7

North Wall Slope Failure and Close-Up of East Limit Looking South Approx. Nov. 12,1979 - After Failure NOTE: For Scale, Observe the Tractor Tracks, Foreground of Plates 6 & 7

APPENDIX A

FIGURES



1014A




MOUNTAIN PARK FORMATION

Massive grey green sandstone separated by dark green siltstones and chert pebble lenses chlorite cement.

LUSCAR FORMATION

MEMBER 'D'

- Grey green sandstone at top of member. Thick sandstone and shale beds common in lower half. Coarsening downwards of grain size in zone dominated by thick basal sandstone, which are massive and well bedded, cemented by kaolinite or illite.
- Thin, dark grey fissile siltstone separated by sandstones.
 Thin coal or carbonaceous shale laminations occuring in above siltstones.



 Torrens sandstone. Highly resistant, thin central shale parting. Breaks into boulder sized rectangular blocks.
 Occasional thin chert pebbles conglomerate. Sandstone massive at top, bedded at bottom.

MEMBER 'B'

- Exposed in 50-A-3 pit. Mainly fissile shales coarsening upwards to siltstones then sandstones.
- Fissile shales

MEMBER 'A'

 Thinly bedded medium to fine grained sandstones, fissile siltstones and carbonaceous shales which grade to thin coal seams.

- No exposure

CADOMIN FORMATION

 Highly resistant, ridge forming conglomerate 2 to 6cm, clasts of chert and quartzite in well cemented silica matrix.

FIGURE A. 3 STRATIGRAPHIC COLUMN FOR THE LUSCAR AREA

































APPENDIX B

SLOPE DISPLACEMENT DATA AND PLOTS

TARLE R.I - THREE-DIMENSIONAL SLOPE DISPLACEMENTS, TARGETS 218,228,238,258,268. PAGE 1 OF 2.

Note: Moriz and Vert are Morizontal and Vertical Components in Feet. Acim and Incl are Azimuth and Anclination (from horizontal) Angles in Degrees. Verticed Dischargement and Apriloation are Continued Ath Demonstration.

	Incl	ERA	9.33	-27.63	-40.76	20.01	18.63	11.47	7 90		7.69	25 22	18, 95	23.0	22, 89	21.68	21.51	25.46	26.97	23.48	26.53	26.31	26.89	25.64	26.57	29 62	12:12	27.32	27.59	27.44		76 27	1. P.	01 04	20. 25	à 2	36.00	901-02	41 UL	50 (N	20.75	24.96
26R	Vert	0,000	0.012	-0,056	-0.050	0.067	0.087	0.055	0.047	-0.022	0.942	0.211	0. 0	0.271	0, 318	0.313	0.352	0.468	0.856	é'8id	0, 995	1.042	1.133	1.162	1.240	1.345	175	1.621	1.917	1.857		7 46	1 480	7.77	7.817	7.074	9.167	Bú≱ 8	o15 d	8,69,5	8. 73 J	8,859
	Az i a	ERR	270	281	222	242	251	257	256	745	261	249	245	246	244	241	245	201	241	239	ô\$č	240	239	239	237	238	237	236	237	237	235	511	192 864	727	227	227	227	226	922	226	226	226
-	Hor 12	0.000	0.073	0,107	0.058	9.181	0.258	0.271	0.310	0.301	0.711	9.448	0.603	0.538	0.753	0.812	0.893	0.983	1.682	1.862	1.993	2.107	2.234	2.421	2.479	2.678	2,869	3.138	3.463	3.576	1.544	207-01	11 11	1.504	13.478	13.922	14, 14	14.529	14.699	285.11	15.273	15.366
][lncl	ERR	00.00	5.54	ERR	2 35	7.46	8, 38	96 61-	68, 39	-13.74	-3.60	5.59	2.25	13.04	16.38	10.65	15.15	15.45	ERR	EPR	ERR	EAR	3.55	20.56	17.57	15.15	18.33	17.91	16.91	11.73		12 81	1.08	27.29	EEP	ERR	14.27	12.72	-3.61	ERR	19.68
258 111	Vert	0.000	0.000	0.013	ERR	0.006	0.025	0.019	-0.077	0.439	-0.045	-0.020	0.018	0.012	0.063	0.082	0.082	0.075	0.094	ERR	EPR	ERR	ERR	0.030	0.120	0.101	0.089	0.197	0.107	0. E07	0.087	0.005	V. 124	0.018	0.132	80.00 10 10	ERR	0.088	0,038	-0.020	ERR	6,120
	8210	EPR	300	209	ERR	286	279	283	278	282	396	247	270	259	267	254	252	264	259	ERR	ERR	ERR	ERR	256	260	259	259	262	261	261	252	1074	266	257	255	9 9 9 9	ERR	253	247	257	ERR	269
	HOF 1 Z	0.000	0.060	0.134	EPR	0.146	0.191	9.129	0.212	0.174	0.184	9.318	0.184	0.306	0.272	0.279	0.436	0.277	0.340	ERR	843	599	ERR B	0.484	0.320	0.319	0.325	0.323	0.331	0.352	0.419	017-0	0.780	0.334	0.322	643	ERR	0, 346	0.340	0.317	EK9	0.355
]	lac!	ERR	9.69	0.00	58, 39	21.25	14 44	7.98	4.76	10.78	16.89	-5.37	18.34	8.49	32.44	16.93	8 . 46	23.58	Z8.77	3.81	17.66	13.96	20.00	15.15	19.80	18.88	9.75	20, 19	22.78	19.91	S - 1	1 2	12.55	6	29.10	60 7 42	18. 17	0 t 6	17.56	- 25.67	17.65	8, 38
238 1111	vert	0.000	0.007	0.000	0.013	0.042	0.042	0.015	0.016	0.028	0.048	-0.017	0.063	0.030	9.143	0.070	0.044	0.103	0.151	0.018	6,0,0	0.084	0.111	0.078	0,079	0.105	0.097	0.111	0.118	0.119	0.058	-0.033	0.077	-0.027	0.172	0.178	0,085	0.053	0.125	-0.148	0.077	0.051
		ERR	297	301	0B]	253	270	270	273	292	287	251	254	264	263	257	396	270	261	260	257	278	263	269	247	262	264	261	245	254	259	107	276	258	257	247	249	240	255	252	257	212
1	21.104	0.000	0.011	0.074	0.008	0.108	0.119	0.107	0.192	0.147	0.159	0.181	0.190	0.201	0.725	0.230	0.295	0.236	0.275	0.270	0.311	0.338	0.305	0.362	0.275	0.307	0.279	0.277	0.281	0.334	0. 28/ 0. 205	547 0	0.346	0.359	0.309	0.263	0.259	0.320	0.395	0.308	0.242	0.346
	1361	ERR	30.02	24,96	-33.02	38,99	31.75	35.33	12.46	14.15	44.10	23.47	27.89	26.47	34.02	27.32	26.57	21,75	30.29	27.52	30.63	27.73	30.74	25.66	29.75	27.20	33.14	31.50	30.47	29.46	28.25	71.95	22.23	27.92	28.45	30.54	30.82	32.26	30.18	29.29	24.80	24,04
28 Vart	YELL	0.000	0.026	0.025	0.013	0.085	0.112	0.151	0.055	0.061	0.281	0.188	0.253	0.253	0.376	0.313	0.333	0.372	0.497	9.459	0.543	9.512	0.549	0.466	0.531	0.499	0.581	0.576	0.569	0.543	0.374	0 255	0.389	0.479	0.518	0.557	0.698	0.582	0.560	0.557	0.468	9.435
TARGET 2	1 29	ERR	207	329	135	265	254	254	255	267	253	242	241	241	243	240	247	243	244	243	242	244	243	246	240	243	243	241	2	245	245	246	246	241	240	239	239	237	243	249	238	244
lari.	2 T 10H	000.0	0.045	0.056	0.920	0.105	0.181	0.215	0.249	0.242	0.290	0.433	0.478	0.508	0.557	0.606	0.665	0.707	0.851	0.881	0.917	0.974	0.923	0.970	0.929	179.0	0.830	0.940	0.967	0.962	001 1 001	1 073	0.952	0.901	0.955	0.244	61ý.J	0.922	0.963	9.993	1.013	0.975
) [] [1361	ERR	14,18	1.97	-38.78	12.32	17.78	16.81	2.13	13.07	12.56	13.53	21.43	18.12	22.53	19.3B	19.76	24.03	20.06	16.71	19.83	17.57	21.14	17.66	18.19	18.24	21.21	21.45	23.55	21.19	13.86	10.01	10.05	15,09	23.13	20.32	21.85	23.67	23.63	20.92	12,21	12.61
18 Vart	Jax	0.000	0.023	0.002	-0.045	0.031	0.059	0.058	0.008	0.052	0.051	0.083	0,137	0.124	0.180	0.153	0.187	0.214	0, 203	0.169	0.212	0.177	0.232	9.198	0.205	0.20	0.210	0 22	0.265	0. 25H	0 217	0.144	0.115	0.171	0.258	0.204	0.261	0.238	0.245	9.232	0.132	0.123
IARSET 21 Ori#	# (7H	ERR	26	261	202	249	252	264	250	262	275	255	254	260	251	255	191	260	258	258	252	260	252	256	22	259	252	254	257	198	C) 85	157	997	25B	255	25.3	248	249	247	222	248	253
[Hariy	71 100	0.000	6.69	0.058	0.056	0.142	0,184	0.192	0.215	0.22	0.229	0.345	0.349	9.379	0.434	0.435	0.526	0.480	0.556	0.563	0.588	9.622	0.400	0.622	0.62	0.619	0.511	9.570	0.698	0.619	10.0	0.478	0.647	0.534	9.604	0.551	9.451	0.543	0.560	0.407	9.619	0,550
[TARGET 218] { ac Dave Waris Asia Vort Tarl	c (po	65.50	87.50	14.50	28-50	14,50	162.45	184.44	190.42	ZEL.41	225.43	239.42	247.44	253.44	277.54	282.52	288.56	295.54	302.58	304.58	305.57	306.58	397.58	308.43	309.52	110.51	311.56	312.56	51.515 51.515	315.67	314.71	314.83	315.25	315.88	316.38	317.56	318.52	519.69	379.58	121.67	322.61	323.25
8 	1/ 17	12.00	12.00	12.00	12.00	12.00	10 4 5	10.30	0ġ∙û	9.50	10.15	10.00	10.30	10, 30	13.60	12.39	13, 30	13.00	14,00	(0. ‡]	13, 45	14.00	11.00	10.15	12.30	12.15	92 I I	11.30	5 ; 1	9.9	12.66	20.60	6.00	21.00	0 ⁰ 6	13, 30	12, 39	15.30	14, 09	je vi	{4 45	6,90
atel		Mar ()6 79	38	2	6	24	Ξ	*? \$	ç.	22	2	27	ŝ	ŝ	÷.	14- 14-	5	4	27	5	Ð	2	÷	ŝ	ц Т	ų,	e .	8	3 5	5 2	May 15 79	1	Ξ	Nev 11 29	2	<u>*</u>	<u>د</u>	Mav 15 79	-0 	<u>.</u>	Mov 18 79	62 81 ×64

TABLE R.I. - THREE-DIMENSIONAL SLOFE DISFLACEMENTS, TARGETS 218,228,258,268. PAGE 2 OF 2.

[0,0,1], (1,1)

Noie: Horiz and Vert are Horizontal and Vertical Components in Feet. Azim and Incl are Azimuth and Inclination (Hrom horizontal) Angles in Degrees. Vertical Displacement and Inclination are Positive in the Rowmard Direction.

2

	Incl	60 QC	20 - 12 27 - 25	10 21	51 02	10 01	20 50 20 52	12.02	Z0. 14	20.02	10 02	11.02	70.05	10 74	30.79	30.28	29,91	30.21	30.25	30.20	30.19	30.03	30.29	30.15	30.20	30.26	30.19	30°37	30.71	20.21	30.26
248	Vert	070 D	0 10 C	C78 6	145 0	0 611	0.0	107.01	10.367	10.447	10.497	10.835	1.174	1 5 4	11.749	12.945	12, 124	12.503	12.749	12.875	13.021	13.018	13, 124	13.024	13.112	13.170	13.157	12.174	13, 137	13,143	191.163
149651 7		726	224	724	776	376	275	275	225	275	275	225	225	275	225	225	224	224	224	224	121	224	\$14	224	122	224	222	224	224	224	274
	Horiz	15 50	14.089	14. 237	16.487	17.184	17.785	17.653	17.853	18, 687	18.145	18 636	19.306	10 701	20.114	20.622	21.072	21.475	21.864	22.129	22.385	22.521	22.470	22.511	22.531	22.570	22.617	22.482	22.559	22.577	22-563
 [Incl	7.14	20.17	14.73	14.19	FRE	-14,34	22.27	14.78	9.72	6.96	14.15	ERR	ERR	EPP	12.43	-5.71	9.20	4. 86	15.54	ERR	ERR	ERR	-7.38	-2.17	8.15	10.50	12.53	2.79	5.07	10.89
258	Vert	0.017	0.137	0.082	0.088	588	-0.99	0.120	0.107	0.062	0.043	0.088	EP.R	ERR	ERR	0.069	-0.033	0.074	0.024	0.094	ERP	ERR	ERR	-040	-0.015	0.049	0.101	0.094	0.018	0.013	0.075
149651 25		755	752	258	260	599	250	259	255	245	260	259	ERR	ERR	ERR	259	250	242	242	260	ERR	ERR	ERR	246	241	244	682	261	270	280	270
	Hariz	0.291	0.349	0.312	0.348	ERR	0.356	0.293	0.406	0.362	0.352	0.347	EPR	ERR	ERR	0.313	0.330	0.457	0.282	0.338	EPR	EPR	ERR	0.309	0.396	0.342	0.545	0.423	0.369	ê. 185	0.390
} [[nc]	84°08	2	30.63	24,00	13.12	-0.40	28.52	19.18	12.38	20.40	15.19	17.86	12.98	19,53	19.29	-5.26	16.93	18.82	28.73	7.74	18.97	11.02	2.36	4.89	17.52	.53	18.80	- 81	10.77	19.76
æ	Vert	0.079	0.099	0.151	0.138	0.086	-0,002	0.144	0.112	0.079	0.119	0.085	0.106	0.065	0.127	170.0	-0.028	0.098	0.992	0.165	0.070	0.067	0.066	0.013	0.026	0.107	0.085	0.111	0.031	0.085	0.072
1AR6E1 238	Aziæ	255	255	250	260	261	253	261	260	260	256	257	25B	258	243	260	246	242	237	258	245	253	258	242	243	241	279	266	271	280	269
1	Hor i z	9.315	0.317	0.255	0.310	0.369	0.284	0.265	0.341	0.360	0.320	0.313	0.329	0.282	9.358	0.269	0.394	0.322	9.279	0.301	0.515	0.385	0.339	0.316	0.304	0.339	0.512	0.326	0.362	0.447	0.379
-] [lncl	25.51	27.87	30,52	28.23	29.74	27.40	31.12	29.37	29.25	28.04	27.57	27.64	30.25	31.68	28.65	21.91	28.21	27.54	39.16	23.30	21.32	Z8.01	24.50	25.19	27.45	26.03	28.33	26.32	25.68	30.82
65.	Vert	9.461	0.525	0.550	0.518	0.564	0.506	0.576	0.570	0.578	0.538	0.519	0.520	0.557	0.569	0.531	9.411	0.552	0.520	0.577	0.490	0.419	0.539	9.462	0.175	0.546	0.565	0.564	0.513	0.526	0.565
TARGET 229	Azia	240	241	238	240	240	237	241	242	236	241	239	235	241	240	2415	239	240	235	240	2.39	237	23B	236	236	236	250	244	243	248	244
	Horiz	996.0	9.993	0.933	0.765	0.987	0.976	0.954	1.013	1.032	1.019	0.994	0.993	0.955	0.922	0.972	1.022	1.029	0.997	0.993	1.139	1.071	1.012	1.014	1.919	1.051	1.157	1.946	1.0.1	1.094	1.060
-) [lac]	21.14	22.89	25.95	24.61	22. 23	ERR	26.08	24.68	EPR	18,69	20.63	18.49	20.74	23.57	19.76	to, 78	17.77	20.91	22.23	19.16	17.30	20.93	14.52	17.24	23.85	19.35	21.44	15.35	9	18.97
80	Yert	9.225	0.266	0.257	0.279	0.275	ERR	0.278	0.272	ERP	-0, 205	0.233	0.205	0.239	0.281	0.218	0.126	0.207	0.233	0.253	0.220	0.208	0,239	0.160	0.185	0.282	0.281	0.258	0.191	0.265	0.254
TAR6ET 218	Azim	251	254	253	255	251	ERR	252	254	893	252	251	224	254	248	256	251	250	250	250	252	247	253	246	250	246	264	260	262	267	259
	Horiz	0.582	0.630	0.528	0.609	0.673	ERR	0.568	0.592	ERR	0.612	0.619	9.613	0.631	0.644	9.607	9.662	9.646	0.610	0.619	0.633	0.668	0.625	0.618	0.596	0.638	0.800	0.657	0.696	0.751	0.739
	Days	324.25	326.27	327.25	328.27	331.27	332.25	334.46	335.25	336.25	337.25	340.32	3+4.63	347.25	350.23	354.31	357.33	361.42	365.33	368.54	372.38	375.31	3 91.4 0	386.46	391.92	396.44	400.44	411.38	416.42	\$21.40	425,54
	li æe														5.30																
	Eate	Ċ,	53	2	r,	27	28	1	Ξ.	e.	3	9	<u>_</u>	<u> </u>	0ec (è 79	2	2	2	1	2	3	<u> </u>	-407	ñ	25	2	Ŧ	2	ŝ	ŝ	2

TARLE 8.2 - THREE-DIMENSIONAL SLOFE DISPLACEMENTS, TARGETS 378, 388, 399, 408. PAGE 1 DF 2.

Note: Horiz and Vert are Horizontal and Vertical Somponents in Feet. Azim and Nocl are Azimuth and Enclination (From Horizontal) Angles in Begrees. Vertical Displacement and Inclination are Fositive in the Bommard Direction.

		lacl	ERP	27.96	28.39	22.18	20.86	17.07	20.08	18.60	17.46	17.80	17.95	17.83	14.88	14.64	14.68	H.58	11.94	14.93	14.93	14.79	14.91	14.72	14.19	11.87	11.59	14.72	14,78	14.76	ERR	14,49	18.83		14.68		20-13	18 7.8	14.78	14.53	14.21	12.11	14.42
:	1 (FB	Ver t	0.000	0.112	0.200	0.212	0.263	0.269	0.402	0.450	0.544	0.679	0.737	1.148	4.325	4.275	4, 331	1.37	4.510	1.61	4.705	1 , 795	4.912	1.949	5.037	5.119	5, 135	5.351	5, 155	5.521	ERR	5.720	5.074	070-0	6. 9.58 4. 007	1 375	4 504	1 100	469 Y	6, 908	6.867	7.179	7.282
		Aziæ	ERK	220	228	214	215	204	9ú2	296	203	204	206	204	641	199	199	199	661	199	199	199	66	66	661	599	661	66 l	66	66 I	ERR	66	66	5	661	001	55	201	661	200	641	200	199
	1 2 2 4 4	Horiz	0.000	0.211	0.370	0.520	0.69.0	0.876	1.100	1.337	1.739	2.115	2.275	3.570	16.274	16.360	16.531	16.802	16.907	17.308	17.642	18, 167	18.446	18,842	19.918	19.347	19.726	20.372	20.669	20, 954	883	22.139	22.561	22.043	22, 757	77 807	24.847	75 754	25.354	26.647	27.205	27.731	28.316
,		laci	ERR	36.53	35.15	33.02	31.36	31.65	31.58	32.31	32.07	31.34	31.46	30.67	31.62	31.45	31.46	31.47	31.59	31.53	31.56	31.51	31.59	31 16	31.45	31.59	31.41	31, 54	31.53	31.58	ERP.	6.89	31-58 31-58	11.47	51.48 11 10			71.47	31.40	31.39	31.13	31.33	31, 30
	398	Ver t	0.000	0.209	0.357	0.468	0.579	0.749	0.909	1.123	1.111	1.691	1.826	2.713	11.737	11.744	11.806	11.933	12.116	12,342	12.557	12.824	13.036	13.206	13,409	13.619	13.726	14.184	14.351	14.536	ERA	ERR	15.462 • • • • • •	13.303	13.014	14 120	18, 701	14.954	17.284	17.596	17.488	18.156	18.380
		₽zi#	ERR	218	222	209	213	20 6	206	206	205	205	206	504	202	202	202	202	202	202	201	201	201	202	202	202	10Z	201	201	201	ERR	ERR	102	107	102	202	202	202	201	202	201	202	202
		Horiz	0.000	0.270	0.507	0.720	0.950	1.215	1.479	1.776	2.252	2.777	2.985	€/2°}	19.062	19.200	19.294	19.498	19,704	20.114	20.447	20.915	21.197	21.587	21.928	22, 146	22.476	23.114	23.392	23.651	84B	ERR	Z5.153	217 36	75 873	24.46A	27.320	27.698	28.313	28.842	29.283	29.822	30.234
		Incl	ERR	35.78	23.22	25.91	25.48	25.62	23.33	23.13	24.47	23.68	24.05	ERR	23.18	23.08	73.19	23,08	23.20	23.28	23.37	23.32	23.32	23.10	23.15	23, 38	23.17	23.48	23.38	23.42	ERR	23.02	25.58	00°07	23.24	15.72	23.61	23.47	23.64	23.50	23, 28	23, 51	23,59
e	202	Vert	0.000	0.147	0.166	0.238	0.304	0.398	0.439	0.516	0.705	0.835	0.912	ERR	5.961	5.992	6.043	6.105	6.206	6.375	6.52 €	6.712	6.814	6.888	7.044	7.185	7.250	7.599	7.672	7.792	ERR	H. ()84	8,43/ B 570	0.000	8.592	R. 951	9.354	9.522	9.814	10,030	19,135	10.467	10.723
		Azie	ERR	218	234	217	221	211	213	215	213	213	213	ERR	207	207	207	Z07	207	207	207	207	207	202	207	207	206	207	206	207	ERR	907	7017 2017	2015	204	206	296	206	206	206	206	206	206
		Hor i z	000.0	0.204	0.387	0.490	0.638	0.839	1.018	1.208	1.549	1.904	2.044	ERA	13.919	14.06	E4. 108	14.327	14.478	14.819	15, 101	15.573	15.804	16.150	16.472	16.616	14 941	17.494	17.750	17.992	EPR I	976.41	14. 3/8	700-11	20.610	20.580	21.396	21.932	22.416	23.064	23.554	24.960	24.551
		latl	ERR	34.66	26.68	26.93	21.13	22.22	23.05	23.86	23.04	23.46	23.35	22.45	23.82	23.58	23.56	23.70	23.94	24.04	24.21	24.11	24,10	24.05	24.01	24.03	24.11	24.18	24.32	24.34	24.39	10.47	74 34 71 36	91 12 91 12	24.47	24.49	24.58	24.53	24.76	24.73	24.64	24.76	24.96
42.1		Vert	0.000	0.204	0.231	0.408	0.412	0.540	0.679	0.880	1.649	1.510	1.398	2.945	9. 199	9.464	9.518	9.707	105-6	10,158	10.409	10.677	10.823	11.923	11.200	11 299	11.519	11.917	12.151	12.320	12.785	1/1/12-21	107.01	17.469	13,664	120.41	14.608	14.703	15.351	15.697	15.918	16.324	16.750
. 133471		Azie	ERR	216	273	214	218	211	213	213	212	212	213	211	209	203	21)9	203	203	20F	209	208	208	209	208	203	208	208	208	208	602	907 907	8/17 8/17	208	208	208	298	208	208	208	297	208	201
1		Horiz	0.000	0.295	0.579	0.803	1.066	1.322	1.643	1.990	2,467	3.013	3.239	4.939	21,514	21.692	21.827	22.118	22.305	22. 774	23, 149	23 857	24 191	24 698	25.147	25, 339	25.734	26.539	26.897	27.240	28.512	100-07	29 460	79.760	30,019	30.791	31.947	32.657	33.316	34.685	34.764	35, 395	199.55
. –	•	Days	304.58	503.57	306.58	307.58	398.43	309.52	310.51	311.56	312.56	313.35	313.69	314.38	314,71	31.83	315.25	315.88	316.38	317.56	318.52	319.69	320.58	321.67	322.61	323.25	324.25	326.27	527-25	528.27	77.165	197700 218 122	335.25	376.25	337.25	340.32	544.63	347.25	350.23	354.31	357-33	54.165	365, 33
	i	1.49	14.00	13,45	14.00	14.00	10.15	12, 30	12.15	13.39	13, 39	B. 3A	窝 <u>-</u> 2	60'4	17.00	20.00	6.00	21.90	6	11 30	12.30	16.30	11.00	16.00	14.45	60°9	66°9	9.30	(iú - 9	1 1 1	27 - 6 - 6	00 11 11 00	60.11 6.60	6.00	6.00	7.45	15. (ii)	6.00	5.39		10 B	00'6ł	e G
		Date	5	5	3	5	3	2	Ê	E.	3	÷.	\$	Ť.	-	<u> </u>	-	_	<u>01</u>	12	-	ŝ	2	<u></u>	<u> </u>	<u>-</u>	81	23	5 8	t (5 8	s e	(1) 20 17 [pr i] 79	Ş	2	99	61	÷	13	4			5

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TARLE 0.2 - THREE-BIMENSTONAL SLOPE DISFLACEMENTS, TARGETS 378, 388, 399, 408. PAGE 2 DF 2.

Wate: Horis and Vert are Horisontal and Vertical Components in Feet. Azim and Incl are Azimuth and Inclination (From Aprisontal) Angles in Degrees. Vertical Displacement and Inclination are Fositive in the Downward Direction.

[ncl	14.53 14.64 14.03 ERR
laber 408 Azim Vert	7.436 7.508 7.476 ERR
TARGET (Acia	200 200 200 ERR
] [Incl Hariz	28,684 28,945 29,908 29,908
] acl	31, 38 31, 36 Erre Erre
TAREET 398 Azim Vert	18.638 18.788 588 588 588
TARGET (Azim	202 202 688 688
}[lach Horiz	30.558 30.824 ERR ERR
] [act	23.59 23.65 23.57 23.64
rARGET 30% Aziæ Vert	10.874 10.994 11.002 11.069
TARGET 2 Azie	206 206 206
Incl Horiz	24.899 25.107 25.223 25.289
Incl	24.85 24.94 24.89 24.89 ERR
fattef 378 Azim Vert	16.856 17.100 17.097 EPR
TAPBET 2 Azim	207 208 207 287
l Hariz	36.429 36.768 36.846 56.846 E88
l Vays	368.54 372.58 575.31 381.40
î i ae	13.00 9.00 7.30 7.30
Dete	Jan 03 99 Jan 07 80 Jan 10 89 Jan 16 80

IARLE B.3 - FIREE-DIMENSIONAL SLOFE DISPLACEMENTS, TARGETS 428, 438, 448, PAGE 1 DF 1.

We have a second

 $M = \{i_1, \dots, i_n\} \in \{i_n, \dots, i_n\}$

Mote: Horiz and Vert are Horizontal and Vertical Components in Feet. Azim and Incl are Azimuth and Inclination (from horizontal) Angles in Degrees. Vertical Displacement and Inclination are Positive in the Domnward Direction.

[Incl	ERR	-20 42	-81.67	-60.26	-54,78	8.88	31.61	0.00	45,00	-4.24
8	Vert								0.000		
TARGET 4	Azia	688	273	08	ť.	=	128	337	270	288	296
1	Her i z	0.000	0.094	0.006	0.020	0.024	0.032	0.039	090.0	0.023	0.081
] [Inc l	ERR	-25.71	- 36.72	10.41	-38.53	6.67	EFR	ERR	ERR	ERR
3B	Vert	0.000	-0.013	-0.074	0.009	-0.043	0.011	ERR	EAR	ERR	ERR
144661 438	Azi a	ERR	251	103	130	83	108	ERR	ERR	ERR	ERA
	Horiz	0.000	0.027	0.126	0.049	0.054	0.094	EPR	ERR	ERR	ERR
} [Incl	ERR	-28.56	-15.42	-10.69	-7.21	-8.21	-9.26	-10.71	ERR	ERR
42B	Vert		•		•	•		•	-0.229		
1ARGET 4		ERR	229	187	195	192	192	194	198	ERP.	EPR
}	Horiz	0.000	0.158	Û. 30	9.461	0.632	0.853	1.078	1.211	ESR	ERR
	₽,95	307.58	398.43	309.52	319.51	311.56	312.56	313.35	313.69	31. VB	314,71
	T are	11.00	10.15	12.30	12.15	13.39	13.30	8°.30	16.30	Jú ь	17.60
	Date	Nov 03 79	Nov 04 79	Rov (15 73	May 06 79	Nov 07 79	Nov (8 79	Nov 09 79	Nov 65 79	Nov 10 79	Nev 10 79

TABLE B.4 - DRE-DIMENSIONAL SLOFE DISFLACEMENTS, SLOPE DISTANCE COMPONENT, PAGE 1 OF 4.

Note: Slope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the Target.

804 (719.668 Dct3179		588	893 1	ERF	SRP.	EPR	8H3	ERR	ERP.	EFR	ERR	ERR	ERP	ERR	EAR	EPP	883	ERR	ERP	ERR	710.668	719,624	710.582	719.530	/19.481	710.423	110 140	710.054	710.096	709.621	706.130	706, 037	745.247	202 202	CH3 . Cill	795.840	795,835	705,891
865	713,482 Oct3179		ERR	ERP	EFA	883	5.P5	ERP	ERR	EPP	EPR	589.3	9.93 199	EP.P.	ERR	ERP	EPR	Εŀ	EFS	ERA	ERA	713.482	713.360	713.305	713.236	115-165	713.085	10 TO TO	712-625	712.566	712.098	708.149	708.054	707 988	001 101		707.870	707.828	707.813
388	738.221 Oct3179		ERA	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	EPR	ERR	ERP	ERR	ERR	EAR	738.221	738.159	738.122	738.075	750-857	737.970		ERR	ERR	ERR	734.289	734,203	741 177	BL3 - 101	154.065	734.037	733.995	734.000
setres) 378	743. 161 Dc t 3179		EPR	ERR	EPP	ERR	ERR	ERR	ERR	ERR	ERR	ERA	ERR	883 8	ERA	ERR	ERR	ERR	ERR	883	ERR	743.161	743.071	743.018	742.930	142,850	742.765	201 120	742.269	742.204	741.694	737.125	724.985	115 712	77, 505	1.56.805	736,767	736.713	736.702
FANCE (in 268	799.472 Mar 0679		799.472	199.469	799.463	799.453	799.460	799.456	799.464	799.425	814.997	121.991	799.413	799.421	799.365	799.314	799.282	799.260	799.247	799.221	799.028	798, 983	798.942	798.906	798,850	778-84J	798.795	107 001	798,519	798.487	798.205	219.2912	795.712	795 479	705 201	193,616	795.591	795.557	795.551
SLOPE DISTANCE (in metres) 258 269 378	800.743 Har 0679		800.743	ERR	ERR	800,742	800.734	ERR	800.734	800.723	800.724	899.727	800.712	800.718	800.657	800.670	800.486	809.672	800.630	809.682	800.662	EB9	ERR	809.656	800.630	899.623	800.668 800.447	100 - 640	883	ERR	ERR	548	693	445	2 2 2	E E E E	EFE	ERP	ERR
238	838, 389 Kar 0679		838, 380	838.368	838.377	838.378	838.370	838.380	838.375	838.354	838.358	838.363	838.357	838.367	838,334	838.337	838,330	838.325	838.316	818.334	838.317	838, 318	838.305	838.310	B39.313	838, 508 331 (1)	838.306 939 311	110,000	ERB	ERP	ERR	ERR	FRR	101	100	ERK	ERR	ERR	ERR
228	801,376 Mar 0679		807.376	807.379	807.376	807.382	807.394	807.375	807.368	807.354	807.340	807.340	807.331	807.330	807.257	807.239	807.223	807.207	807.201	807.184	807.146	807.136	807.124	807.126	807.123	90/.119 717 119	807.117 807.117	2011	883	ERR	ERR	ERR	523	245	445	144	EKH	EFR	843
[218	844.771 Nar 0679		844.771	844,756	844.753	844,758	844.765	844.755	844,755	844.740	844.734	844,733	844.725	844.725	844,691	844.670	841,665	844.670	844.652	844,666	814.648	844.648	811.679	844.642	844.630	844,630 544,630	844.625 BA4.477	127 180	844,635	844,635	844.630	844.629	844.636	244 447	240-140	t H H	844,627	ERP	ERR
+] 408			ERR	ERA	ERR	ERR	ERR	ERR	ERR	EPR	ERR	EPR	ERR	ERR	59R	ERR	ERR	ERR	ERR	ERR	ERR	0.00	0.144	9.282	0.453	9.61	0.804	1 477	2 014	2.172	3.435	14.888	15.210	15 474		15.692	15.774	15.856	15.969
398			ERR	EP.R	ERR	ERR	ERR	ERR	ERR	ERR	EAR	EAR	ERR	ERR	ERR	ERR	ERP	ERR	ERR	ERR	EPR	0.000	0 tú	0.581	0.807	1-0-1	1.316	10011	2.812	3.005	4.541	17.497	17.808	8 075	C77.0	8.31/	8,412	8.550	8.597
																																-	-				-		
feet) 388			EPR	ERR	ERR	ERR	ERR	ERR	ER8	EPR	ERR	ERP	EPR	ERR	ERR	0.000	0.203	0.325	0.474	0.420	0.823 1 607			ERR							_		13.848						
MEWT (in feet) 378 388			ERR ERR													ERR ERR					EAR EAR	0.000 0.000					1.299 0.823 5 508 5 AV7	EDD	EBB		ERR	12.900	13, 192	11 149		15.642	13.727	13.865	21.191 13.848 1
. DISPLACEMENT (in feet) 268 378 388							ERR				ERR				ERR		ERR	ERR					0.295	0.457	0, 758	020		7 ANS EDD	2.926 ERR	3.140	4.813 ERR	19.803 12.900	20.267 13.182	20 505 13 349		29.855 15.642	20.977 13.727	21.155 13.865	-
(JMULATIVE DISPLACEMENT (in feet) 258 268 378 388			0.000 ERR	ERR	0.030 ERR	ERR	0.040 ERR	ERR	0.092 ERR	0.155 ERR	0.17B ERR	0.168 ERP.	ERR	0.168 ERR	0.351 ERR	ERR	0.624 ERR	0.696 ERR	0.739 ERR	0.824 ERR	1.457 ERR	1.605 0.000	1.739 0.295	1.857 0.469	2.941 0.758	2.133 1.029	1.299		3.127 2.926 ERR	3.232 3.140	4.157 4.813 ERR	12,008 19,803 12,900	12.336 20.267 13.182	52 344 70 505 17 749		12-651 20.855 15.642	12.733 20.977 13.727 1	12.845 21.155 13.865 1	21.191
JEMENT (in feet 378			0.000 ERR	0.010 ERR	0.030 ERR	0.06J ERR	0.030 0.040 ERR	0.053 ERR	0.092 ERR	0.155 ERR	0.063 0.17B ERR	0.168 ERP.	0.194 ERR	0.082 0.168 ERR	0.283 0.351 ERR	0.519 ERR	0.187 0.624 ERR	0.233 0.696 ERR	0.371 0.739 ERR	0.201 0.824 ERR	0.266 1.457 ERR	1.605 0.000	ERR 1,739 0.295	0.286 1.857 0.457	0.371 2.941 0.758		2,221 1,299 2,302 1,598		ERP 3.127 2.926 ERR	ERR 3, 232 3, 140	ERR 4.157 4.813 ERR	ERR 12,009 19,803 12.900	ERP 17.336 20.267 13.182	1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1		ERR 12.651 20.855 15.642	ERR 12,733 20,977 13,727 1	ERR 12.845 21.155 13.865 1	12.864 21.191
CUMULATIVE DISPLACEMENT (in feet 258 268 378			0.000 0.000 0.000 ERR 1	0.040 ERP 0.010 ERR 1	0.010 ERR 0.030 ERR	0.006 0.004 0.063 ERR	0.032 0.030 0.040 ERR	-0.001 ERR 0.053 ERR	0.916 0.030 0.092 ERP	0,085 0,066 0.155 ERR	0.072 0.063 0.178 ERR	0.055 0.053 0.168 ERR	0.075 0.102 0.194 ERR	0.042 0.082 0.168 ERR	0.150 0.283 0.351 ERR	0.141 0.240 0.519 ERR	0.163 0.187 0.624 ERR	0.180 0.233 0.696 ERR	0.209 0.371 0.739 ERR	0.150 0.201 0.824 ERR	9.206 0.266 1.457 ERR	0.203 ERR 1.605 0.000	0.246 ERR 1.739 0.295	0.229 0.286 1.857 0.469	0.219 0.371 2.941 0.758	0.256 0.394 2.153 1.020	0.246 2.221 1.299 0.250 2.221 1.299		ERR ERR 3.127 2.926 ERR	EFR ERR 3, 232 3, 140	EPR ERR 4.157 4.813 ERR	ERR 12,008 19.803 12.900	E8P E8P 12,336 20,262 13,192	TERP FOR 52 MAR 20 545 11 146		EAR EAR 12.651 29.855 15.642	EPR ERP 12,733 20,977 13,727 1	ERR ERR 12.845 21.155 13.865	EFR ERR 12.864 21.191 1
CUMULATIVE DISFLACEMENT (in feet 18 228 238 258 268 378			0,000 0.000 0.000 0.000 ERR I	-0.010 0.040 ERP 0.010 ERP 1	0.000 0.010 ERR 0.030 ERR	-0.021 0.096 0.004 0.063 ERR	-0.660 0.032 0.030 0.040 ERR	0.00Z -0.001 ERR 0.053 ERR	0.025 0.016 0.030 0.092 ERR	0.071 0.985 0.066 0.155 ERR	0.117 0.072 9.063 0.178 ERR	0.117 0.055 0.053 0.168 ERR	0.146 0.075 0.102 0.194 ERR	0.150 0.042 0.082 0.168 ERR	0.389 0.150 0.283 0.351 ERR	0.448 0.141 0.240 0.519 E8R	0.591 0.153 0.187 0.624 ERR	0.553 0.180 0.233 0.696 ERR	0.573 0.209 0.371 0.739 ERR	0.629 0.150 0.201 0.824 ERR	0.753 0.206 0.266 1.457 ERR	0.786 0.203 ERR 1.605 0.000	0.825 0.246 ERR 1.739 0.295	0.819 0.229 0.286 1.857 0.469	0.827 0.219 0.371 2.941 0.758	0.842 0.236 0.394 2.153 1.020	0.242 0.246 2.221 1.299 0.724 0.750 2.337 5.598		ERR ERR 3.127 2.926 ERR	ERR EFR ERR 3.232 3.140	ERR ERR 4.157 4.813 ERR	ERR ERR 12,008 19,803 12,900	ERP ERP ERP 17.336 20.267 13.192		1 101 011 101 111 111 111 101 101 101 1	EPR ERP ERP 12.651 29.855 15.642	ERR ERR ERR 12,733 20,977 13,727 1	ERR ERR 12.845 Z1.155 13.865	ERR EFR ERR 12.864 21.191
СИМИ.411VE DISPLACEMENT (in feet 228 238 258 268 378		Days	0.000 0.000 0.000 0.000 0.000 ERR I	0.050 -0.010 0.040 ERP 0.010 ERP 1	0.060 0.000 0.010 ERR 0.030 ERR	0.044 -0.021 0.006 0.004 0.063 ERR	0.021 -0.060 0.032 0.030 0.040 ERR	0.054 0.002 -0.001 ERR 0.053 ERR	0.054 0.025 0.016 0.030 0.092 ERP	0.103 0.071 0.985 0.966 0.155 ERR	0.123 0.117 0.072 0.063 0.17B ERR	0.126 0.117 0.055 0.053 0.168 ERR	0.153 0.146 0.075 0.102 0.194 ERR	0.153 0.150 0.042 0.082 0.168 ERR	0.264 0.387 0.150 0.283 0.351 ERR	0.267 0.448 0.141 0.240 0.519 E8R	0.349 0.591 0.163 0.187 0.624 ERR	0.333 0.553 0.180 0.233 0.676 ERR	0.392 0.573 0.209 0.371 0.739 E8R	0.346 0.629 0.150 0.201 0.824 ERR	0.405 0.753 0.206 0.266 1.457 ERR	0.405 0.786 0.203 ERR 1.605 0.000	0.467 0.825 0.246 ERR 1.739 0.295	0.425 0.819 0.229 0.286 1.857 0.469	0.464 0.829 0.219 0.371 2.948 0.758	0.454 0.842 0.236 0.394 2.153 1.020	0.848 0.242 0.246 2.221 1.299 0.841 0.724 0.250 2.497 5508	0.145 EDD EDD 2.200 2.447 2.000	0.449 ERR ERR 3.127 2.926 ERR	0.448 ERR EFR ERR 3.232 3.140	0.464 EPR EPR 4.157 4.813 EPR	0.457 EPR EFR EFR 12.008 19.803 12.900	0.445 FRP FRP FRP 17.336 20.267 13.182			ERR ERP ERP ERP 12.651 29.855 15.642	0.474 ERR ERR 12.733 20.977 13.727 1	EPR EPR EPR 12.845 21.155 13.865 1	ERR ERR EFR ERR 12.864 21.191
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JARLE R.4 - ONE-DIMENSIONAL SLOPE DISPLACEMENTS, SLOPE DISTANCE COMFONENT, PAGE 2 OF 4.

Note: Stope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the larget.

198561		518	822	CUP 238	KULATTVE 258	CUMBLATTVE DISPLACEMENT (in feet) 258 268 378 3	15NF (in 378	feet) 388	3 6£	80¥ [{} 21B	228	5 238	LOPE D151 258	SLOPE DISTANCE (in 258 268	æetres) 378	388	398	80¥
lero Rezding Lero Keading Bate											844.771 Mar 0679	807.376 1 Mar0679 1	838.380 Har0679	800.743 Mar 0679	799.472 Mar(1679	743, 161 Oct 3179	738.221 Dct3179	713,482 Oct3179	710.668 Dct3179
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62 11		ERR	ERR	ERR	883	12.956	21.315	13.979	18.710	16.079	ERR	ERR	ER9	893	795, 523	135 66	733,960	707.779	705.767
11 79		ERR	ERR	ERR	ERR	12.956	21.374	15.996	18, 737	16.158	ERP.	ERR	ERR	ERR	795.523	736.646	733.955	707 771	705.743
67 11		ERR	ERR	ERR	599	12.969	21 394	14.012	18,740	16.115	ERR	589	ERF	EPR	795.519	736 613	733.950	707.779	112.201
62 11		0.454	883	ERR	893	13.028	21.578	14 127	19.835	16.292	844.633	ERR	558	ERR	795.501	735 584	733.915	107.741	705.702
		104.0	ERP E	EPR 1	525	13.038	21. 470	14.183	18 947	16.296	844.620	89 I	ERR 1	EPR	795.498	736.617	7.33, 898	707.707	705.701
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61 21		ERR	5 6 6	ERE	1 1	13.215	21.739	14.268	19.045	16.397	883	688	ERR	E B B	795.444	736.535	733.872	707.677	705,670
12 79		883	ERR	EFR	EPR	13.199	21.762	14.252	19.048	16.420	ERR	ERR	ERR	ERR	795.449	734.528	733.874	707.676	705.663
12 79		50H	ERR	ERP.	ERP	13.179	21.785	14.334	190'61	16.450	ERR	ERR	ERR	Err	795.455	736.521	733.852	707.672	705.654
12 29		ERR	ERR	EPR	ERR	13, 215	Z1.781	14.327	19,091	16.450	ERR	E P.R	ERR	ERR	195.444	736.522	733.854	707.666	795.654
62 21		0.500	EPR	ERR	ERP	13.416	22.122	14.511	19.337	16.742	844.619	ERR	ERR	ERR	795.383	736.418	733, 798	707.588	705,565
		0.418	ERF 1	ERR	ERR	13.432	22.181	14.537	19.403	16.742	844.644	ERR	E 88	ERR	795.378	736-400	733, 790	707.568	705.565
		9.464	55	EPR	ERR	13.455	22. 21	14.615	19.419	16.781	B44.630	ERR	ERB	ERR	795.371	736.390	733.766	707.563	705.555
			893 191	528	19 19 19	13.452	22.546	14.616	19.442	16.817	ERR	EKR	a a	ERR	795.372	736.289	733.766	707.556	795.512
14 / 4		0.428 0.428	202		200	13. 440 13. 440	22.247	14.810 14 055	17.647	1/. UAB	844-005	111		100	210.071	736, 280 711, 380	207.001	CY1.1U1	105 481
62 H		0.504	568 568	668 101	EBR F	13.704	22.641	14.898	10.747	17.080	841,618	ERR	EBR	288 888	795.295	736.259	733.680	707.463	705.462
ŧ		0.543	883	883	ERR	13.685	22,588	14.885	19.741	17.103	841.606	ERR	EPR	ERR	795.301	736.276	733.684	707.465	705.455
áž Þi		0.467	ERR	993 9	ERR	13.749	22.792	14.944	19.793	17.Zņ8	841.629	ERR	5RR	ERR	795, 284	736.214	733.646	707.449	705.423
4: SI		0.458	ERA	EPR	ERR	13.879	22.962	15.121	19.954	17.395	844.632	ERR	EFR	ERR	795.242	736.162	733.612	707.409	705.366
12 14		9.487	ERP	843	88	13.967	23.120	15.229	20.078	17.483	B44.623	E56	ERR	ERR	795.215	736.11	733.579	707.362	705.339
2: 2		0.500 2.30	588	843	EFR F	14, 108	Z3, 356	15.367	20.243	17.654	844.619	EFR	1993 1993	5	715,177	736,042	155.557	707.512	705.287 *** 267
5 P		778-0 9 441	523 525	120	1973 1973	101.FI	23.443 21 53A	15.53	101.05	17 978	044, 671 844, 671	833 833	1 1 1 1 1 1		745, 135	115.980	733.497	207-101	705, 231
16 79		0.481	EPR	ERR	883	14.347	23.769	15.655	20.561	17.989	844.625	ER9	ERB	E	795.099	735.916	733.449	707.215	705.185
62 EI		0.438	ERR	£28	538	14.528	23. 453	15.797	20.639	18.153	841.638	ERR	ERR	EPR	795.044	735.860	733.406	707.176	705.135
62 II		9 et 1	883 ·	ERR	EPP.	14.577	24, 157	15.945	20.892	18.317	844.637	ERR	ERR	ERP	795.029	735,798	733.361	797.114	705.085
18 <i>7</i> 9		0.458	ERR	ERB	EPR	14.738	24.376	16.155	21.105	18.527	841.632	ERR	ERR	ERR	194°.980	735.731	133.297	701.049	705.021
62 61		9.471	ERP.	588	ERR	14.843	24 596	16.279	21.243	18.691	841.678	ERR	ERR	55	794.948	735,664	732 259	200.707	704.971
0		0.520	583	ERR	6633	15.003	24.813	16.479	21, 394	8.8.8	B44.613	EFF	ERR		794.899	735.598	733.201	706,961	704.923
<u>e:</u>		0.497	888 1	689	62	15.220	25.170	16.719	21.716	261.61	844.620	ERR	ERR	ERA	794,833	735 489	733, 125	705.863	704 8 8
21 79		9.454	E PR	558	66 11	15 107	25, 515	16.958	21,991	17.412	844,633		32		194.776	735.384	753, 052	704 779	704.742
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TARLE 8.4 - CHE-DIMENSIONAL SLOPE DISPLACEMENTS, SLOPE DISTANCE COMFONENT, PAGE 3 OF 4.

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Note: Slope Bistance is the Direct Reading from the Electronic Distance Measuring Instrument to the Target.

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713.425 713.425</th><th>34.171 91.776 34.771 91.776 34.771 91.776 37.472 74.161 73.272 71.38 1 fee 045 14 0.477 66.77 66.77 67.175 74.161 73.273 74.161 73.273 74.161 73.273 74.161 73.273 74.161</th><th>Defining BML/71 BVL/71 BVL/7</th><th>Predicta BML/71 BPL/71 BPL/7</th><th>Prediration BML/T1 BM</th><th>Deterina BML,71 BVL,71 BVL,71 BVL,71 BVL,71 PVL,72 7L,13 7L,14 7L,14</th><th>Distribute BML,71 BML</th><th>District Mit.711 <</th><th>Prediate M4.711 07.35 08.7.31 07.4.13 07.137 07.1</th><th>Petete Phila <t< th=""><th>Preference Method Met</th><th>Printing Multiply markets Multiply markets</th><th>Printer Multiply models Multiply models</th></t<><th>Printing BM./TI BM./T</th><th>Marking Marking <t< th=""></t<></th></th></td<></th></thd131<></thd131719<></thd13179<> | Decision BH4.171 B07.376 B78.360 B00.743 797.472 741.161 733.221 713.42 te 1 ae Days Data Data <td< th=""><th>Diametrical B44.771 B77.371 B77.472 747.161 734.721 734.161 734.221 713.422 714.431 775.442 714.451 755.451 725.451 726.451 756.451 756.451 756.451 756.451 756.451 756.451 756.451 756.451 756.451 756.451 756.451 756.551 756.551 756.551 756.551 756.551 756.551 756.551 756.551 756.551 756.513 756.511 756.551</th><th>D Feading B44.771 B07.376 B33.281 739.472 743.161 739.231 713.421</th><th>5 Feading Feading Beading Beading</th><th>5 Fedirat B44.711 B7.374 B7.374 B7.471 B7.477 74.161 739.221 713.420 5 Fedirat Fraining bate B44.71 B7.4677 Arr6677 Arr6777 Arr677 A</th><th>5 Feddra PH4.771 907.376 584.771 907.376 584.731 791.472 741.41 739.221 711.48 2 Feddra Freadrang Tatte Harrols T Harrols T</th><th>5 Pediat BML 771 BVL 771 BVL 771 BVL 771 BVL 771 PTL 712 <</th><th>D Feddra B44.771 By7.376 B74.771 By7.376 B74.777 By7.377 B74.777 By7.377 B74.777 By7.377 B74.777 B74.877 <</th><th>0. Feedrag B44.771 B74.771 B74.472 74.161 739.472 713.425</th><th>34.171 91.776 34.771 91.776 34.771 91.776 37.472 74.161 73.272 71.38 1 fee 045 14 0.477 66.77 66.77 67.175 74.161 73.273 74.161 73.273 74.161 73.273 74.161 73.273 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161 74.161
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 07.137 07.1 | Petete Phila Phila <t< th=""><th>Preference Method Met</th><th>Printing Multiply markets Multiply markets</th><th>Printer Multiply models Multiply models</th></t<> <th>Printing BM./TI BM./T</th> <th>Marking Marking <t< th=""></t<></th> | Preference Method Met | Printing Multiply markets Multiply markets | Printer Multiply models Multiply models | Printing BM./TI BM./T | Marking Marking <t< th=""></t<> |

TABLE 8.4 - DRE-DIMERSIBHAL SLOPE DISPLACEMENTS, SLOPE DISTANCE CONFORENT. PAGE 4 OF 4.

Note: Slope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the Target.

•08	710.668 Det3179		793.595	93.568	ERR	703.503	103.494	793.460	703.432	203.439	703.418	703.314	703.274	793.275	793.255	703.188	703.150	793, 147	703, 680	793.033	703. 805	186.205	202°926	792.953	792.939	712.917	005.204	702.864
: 865 268	713.482 711 Bct3179 Dc		02-210-20	705.696 79.	883	705.639 70	705.621 70		705.572 70	195.575 70	705.556 70	705.469 70.		705.435 700	705.420 70	702 360 70		•	705.269 79		705.218 70	705,182 70		705 152 70	705.146 79		705.115 70	795.085 70
E n					a۲																							
389	739.221 Dct3179		732.043	732.021	ERR	731.962	731.952	731.930	731.895	731.899	731.875	731.791	731.758	731.756	731,734	731.679	731.652	731.630	731.690	731.542	731.525	731.501	731.479	731.455	731.446	731.429	731.400	731.385
etres) 378	743.161 Oct3179		34.005	733.975	733.943	733.995	733.884	733.851	733.813	733.804	733.773	733.648	733.616	733.608	733.577	733.458	733.463	733.453	EAR	EPR	733.300	733.26≰	733.235	733.215	733.200	33.193	53.426	733-121
SLOPE DISTANCE (in metres) 258 268 375	799.472 7 Mar 0679 0		793.933 7	EPP 7	193.904 7	793,860 7			793.830 7	793.820 7		793.723 7		793.695 7	793.686 7	793.630 7			193.552	ERR	793.506 7	793.484 7		793.454 7	793.437 7	793.422 7	793.393 7	793.381 7
DISTAN 258			ERR 79	EP.R	ERR 79	ERR 79		EPR 79	ЕРР. 79			EPR 79		ERR 79		ERR 79				ERR	ERR 79	EP.R 79	ERR 79	ERR 79		ERR 79	ERR 79	EER 79
	809, 743 Nar 0679																											
234	838,380 Mar0679		ERR	ERR	ERR	ERR	ERK	ERR	ERR	ERR	ERR	ERR	EF.9	ERR	58B	ERR	ERR	EPR	599	EPR	ERR	EPP	ERP	ERR	893	ERP	EPR	ERR
228	807, 376 Kar 0679		ERR	ERR	ERP	ERR	ERR	ERR	ERR	ERR	E 89	993	ERR	ERB	ERR	ERA	ERR	EFR	EAR	ERR	ERR	EPR	ERA	588	ERP	ERR	ERR	EFR
21B	844, 771 Mar ()679		844.620	844.622	844.630	844.616	844.625	844.614	844.529	844.635	844.623	844.627	844,622	844.632	844.638	844.628	844, 620	844.630	844.590	844.617	844.617	844.613	844.609	814.618	844.615	844.612	844.599	844,615
40B 2			23.205	23.294	ERR	23.507	23.536	23.648	23.740	23.717	23.786	24.127	24, 258	24.255	24.327	24.540	24.665	24.675	24.895	25.049	25.141	25, 220	25.302	25.311	25.357	25.429	25.495	25.693
39B			25.478	25.544	ERR	25.761	Z5, 790	25.849	25.951	25.941	26.004	24.319	26.391	26.401	26.450	26.647	26.732	26.752	26.945	Z7.080	27.113	27.231	27.270	27.529	27.349	27.398	27.459	27.546
feet) 388			20.269	20.34I	ERR	20.535	Z0.567	20.610	20.754	20.741	20.820	21.096	21.204	21.210	21.283	21.463	21.552	21.624	21.722	21.912	21.968	22.047	22.119	22.198	22.227	22.283	22.378	22 . 1 29
IENT (i.e. + 378			30.039	30.137	30.242	30.367	39, 136	39.544	30.669	30.698	30.800	31.210	31.315	31.341	31,443	31.834	31.817	31.850	ERR	ERR	32, 352	32.470	32:565	32, 631	32.680	32,736	32.923	32.939
CUMULATIVE DISPLACEMENT (in feet) 258 268 378 3			18.173	ERR	18.269	18.412	18.415	18.481	18.511	18.543	18,612	19.862	18,927	18, 954	18, 983	19.167	19.226	19.242	19.423	ERR	19.574	19.646	19.741	19.744	19.800	19.849	\$\$0.94	19.984
111. AT 1 VE 258			ERR	ERR	ERR	ERR	ERR	ERP	ERR	EPR	ERR	ERR	583	ERR	588 8	ERP	ERR	ERF	689	ERP	ERR	ERR	EPR	666 6	ERR	EPP	ERR	ERR
CU9 238			ERR	ERR	ERR	E P.P.	555	EPR	EPR	843	555 1	ERR	EFR	ERR	588	599	EFR	595	err Brr	843 843	EER	843	55.0	993	599	ERR	553 1673	ERP
822			ERR	ERR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	ERR	ERR	ERR	EPR	ERP	ERR	ERR	583	ERP	ERR	EPR	ERR	ERP	ERR	EPR	58R	EPR
218			0.497	0.490	0.464	0.510	0.481	9.517	0.467	0.448	0.487	0.474	0.490	0.458	0.438	0.471	0.497	0.461	0.595	0.507	0.507	9.520	0.533	9.504	9.513	0.523	9.566	0.513
ٺ		θays	344.25	340.60	341.00	341, 29	341.71	341.98	342.35	342.63	342.93	344.65	344, 98	345.31	345.45	346.63	346.98	347.31	347.73	347.95	349.69	349.98	350.29	350.71	350.96	351.29	351.75	352, 33
	a jeg	li se	6.00	11.30	24.00	1.00	(17.0)	23.30	8.30	15.00	22.09	15.30	23.34	1.30	15.30	15.99	23.30	1.34	17.30	23,00	16.30	23, 30	1.69	(7. 00	23.00	1.09	18.00	8.06
1 ARGE 1	lera Seading Lera Seading Bate	Date		Dec 06 79		Ċ.	6	67	8	98	e.	ŝ	ŝ	1	Ξ	2	2	÷	:::		<u>.</u>	5	£6	ž	18			

TABLE P.S - ONE-DIMENSIONAL SLOPE DISFLACEMENTS AND VELOCITIES, SLOFE DISTANCE COMPONENT, PAGE 1 OF 4.

gionae trafficher L Note: Slope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the Target.

	Velocity Modian	Days		74.00	85.00	00.101	118.50	125.50	152.00	140-00	153.00	159.49	186.93	225.42	246.46	265.50	280,01	285.54	292.03	299. Ĥ	303. SA	395.08	305.97	396.88	347.92	308.98	510.02	361-JT TT2 94	211 4V	51 J. 98	114.48	314,59	314.59	314.62	314.66	311.76	314.92
80	e i	£ -		EPP	EPP				2 C		198	ERR		ERR	EPP	483	EPP				e e e						1,400.0									_	0.0273
39E				ERR	EPR	589	689	188 199 199	1 a a a	111	588	ERR	EPP	843	ERR	EPR	ERR	EPR	843	EPR	893						0.0196				2.8412			.~			0.0121 (
388				EPR	EPR	ERR	ERR	ERR F	, 14 1 1 1		588	ERP	EPR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	EPR						0.00//										- 01-00°0-
COMPONENT VELOCITY (feet/hour) 258 268 378				ERR	ERR	ERR	ERR	EPR	2 0 2 1 0 0 1		885	ERR	55.6	ERR	ERR	ERA	EPP	ERR	EPP	ERR	ERR	0.0124					0.0126	0 0275	0.0405	0.0861	3.2873		0.5058	0.4830			0.008B -
268 268				0000"	0.0002	0.0001	1000-0	0-0001	2000.0	0 0002	-0.0001	0.0002	0000	0.0003	0.0005	0,0072	0.0006	0.0003	0.0005	0.0037							1000 0							0.2871	0.0570		0.9048
HPONENT V 258				ERR			•		ЕКМ Д ДОЛТ			0.0003							-0.0910			ERA				-0.0056					ERR	EPR	ERR	EFR	ERR	EPR	ERR
CU 238				0.0001	0,0002	0000	0.001	-0.0012	0.0007	0.0001		0.0001									0.0001	0.0018					10/10/	001		ERR	ERR	ERR	ERR	EPR	ERR	EPR	ERR
228										0.0004 -		0.0002							0.0003 -					0.0004 -			'	500 500	5RP	ERR	ERR	ERR	5 P.P	5PR	ይያዋ	888 8	EFR
218							•		0000.0						.0000											0.0006		10.00	0.000	0.0008	0.0007	-0.0957	-0.9410	ERR	ERR	EPR	ERR
			ERR	ERR	ERR										ERR	589			ERR					0.453		0.804			2,172					15.672	15.774	15.856	15.968
348			ERR	ERR	ERR	ERR	ERR	trik Link		2 22	883	ERR	ERR	ERR	0,000	0,400	0.581	0.807	1.047	1.316	1.268	010-7	3,005	4.545	17.497	17, 898	18.025	18.317	18,412	18.550	18.599						
feet) 388			ERR	ERR	ERR	ERR	ERR	25 G			688	ERB	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	0.000	0.203	0.325	0.479	0.629	0.823	1.00/	203	ER9	ERR	12.900	13.182	13.369	13.642	13.727	13.865	13.848
CUMULATIVE DISPLACEMENT (in feet) 258 268 378 3			ERR	ERR	ERR	ERR	ERF		144		883	ERR	ERR	ERR	0.000	0.295	0.469	0.758	1.020	1.299	840°.I	100 0	3. 140	813	19.803	20.262	20.505	20.853	29.777	21.155	21, 171						
DISPLACE 268			0.000	0.010	0.030	0.063	0.040	5.05.0 200 0	0.175	821.0	0.168	0.194	0.168	0.351	0.519	0.624	0.696	0.739	0.824	1.457	1.695	1.739	1.857	2,041	2.133	2.221	2 9 2	1012	7.77	4,157	12.008	12.336	12,444	12.651	12.733	12.845	12.964
UMULATIVE 258			0.000	ERR	ERA	0.004	0.030	EHR 2 220	100.0	00110	0.053	9.102	0.082	0.283	0.240	0.187	0.233	0.371	9.201	0.266	59R	E88	0.286	0.371	0.394	0.246	0.02.0	001	643	EPR	ERP	ERR	5RP	ERR	ERR	ERR	ERR
C 238			0.000	0.040	0.010	0.006	0.032	-0-001	0.010	0.077	0.055	0.075	0.042	0.150	0.141	0.163	0.189	0.209	0.159	0.206	0.203	0.246	0.229	0.219	0.236	0.242	07770 CD0		188	EPR	ERR	EPR	EFR	883	ERR	EKR	EFR
22B			0.000	-6.910	0.600	-0.021	-0.060	200.0	120 V	117.0	0.117	0.146	0,150	0.389	0.448	0.501	0.553	0.573	0.629	0.753	0.786	0.825	0.819	0.829	0.842	0.648	U, Bel		686	EPR	EPR	ERR	ERR	EPR	ERR	ERR	ERR
218			0.000	0.050	090.0	0.04	0.021	0.034	101 0	0 121	0.126	0.153	0.153	0.26	0.267	0.349	0.333	0.372	0.046	0.405	0.405	0.467	0.425	0.46	0 46	0 181	0 445	0 468	0.448	0.464	0.467	0.445	0.425	ERR	9.474	ERP	ERR
1		Days	65.50	82.50	87.50	114.50	122.50	0C.B1	02 FF1	149 50	156.50	162.45	211.41	237.42	253.50	277.50	282.52	288.56	295.50	302.58	394,58	305.57	306.36	307.40	308.43	309.52	10.016	52 212	311.57	314.38	314.57	314.58	314.60	314.63	314.69	314.83	315.00
	ing Date	lise	12.00	12.00	12,00	12,00	12.00	12.00	14. UN	12 18	12,60	10.45	9.50	10.00	12.00	12.00	12.30	13, 30	12.00	14.00	14.00	13.45	8, 45	۰°3	[0 .]5	12.39	01-21	5-0 5-0	17.50	60.9	13.45	14.03	14.20	15.90	16.30	29,90	54 QU
135491	lero Reading lero Reading	Date	9 0	23	87	24	May 02 79	<u></u>	27	5 8	5		30	12	ŝ	6	60	<u>در</u>	22	2	77		3	2	Ē	23	2 3	9 9	Ę	<u> </u>	ŝ	ŝ	01	Ê	97 61 VAN	Nov 16 79	Nov 10 79

TARLE R.5 - DRE-DIRENSIONAL SLOFE DISFLACENENTS AND VELOCITIES, SLORE DISTANCE CONFORMAL. PAGE 2 OF 4.

a de companya en companya La companya en c Note: Stope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the Isrget.

	/elocity Moutet	5 APG	315.19	315.44	315,54	315.73	315.92	316.11	316.31	316.39	316.45	316.51	316.54	316.91	317.39	317.55	317.62	317.91	318, 22	318.35	318.48	318.6 0	318, 52	319.30	319.65	320.08	320.34	320.66	321.16	321.62	322.32	322.61	323.12	123.89	324.82	325. t u	325.61	325.84	326.14
80¥ [5	-	0.0122	0,0273	-0,0048	0.0205	0.0017	0.0113	-0.0012	0,0182	0.0159	0.0246	0.0000	0.0169	0.0000	0.0164	0,0376	0.0170	-0.0109	0.0177	0.0129	9,0292	0.0156	0.0148	0.0155	0, 00B4	0.0336	0.0121	0.0145	0.0149	0.0165	0.0152	0.0115	9.0146	0.0119	0.0137	0.0187	0.0095	0.0152
398			0.0122	0.0091	0.0017	0.0132	0.0581	0.0042	0.0273	-0.0023	0.0023	0.0109	0.0410	0.0148	0.0119	0.0969	0.0239	0.0162	0.0041	0.0217	-0.0034	0.0146	0.0134	0.0708	0.0147	0.6197	0.0179	0.0154	0.0113	0.0184	0,0168	0.0128	0.0110	0.0137	0.0132	0,0080	0.0105	0.0137	0.0157
rr) 388			0.0144	0.0057	0.0085	0.0159	0.0290	0.0024	0.0162	0.0182	-0.0046	0.0601	-0.0137	0.0106	0.0048	0.0328	0.000	0.0157	0.0164	0.0105	-0.0068	0.0164	0.0148	0.0180	0.0125	0.0102	0.0221	0.0100	0.0125	0.0134	0.0165	0.0115	0.0139	0.0106	0.0115	0.0108	-0.0032	0.0170	0.0122
(feet/hou 378			0.0137	0.0205	0.0051	0.0269	-0.0564	0.0259	0.0273	0.0114	0.0159	1410.0	-0.0048	0.0197	0.0107	0.0137	0.3452	0.0003	0.0109	0.0169	-0.0290	0.0565	0.0142	0.0262	0.0214	0.0095	0.0273	0.0192	0.0163	0.0184	0.0173	0.0294	0.0158	0.0152	0.0165	0.0174	0.0263	0.0075	0.0187
COMPONENT VELOCITY (feet/hour) 258 268 378			0.0101	0,000	0.0068	0.0082	0.0051	0.0079	0.0186	0,9410	-0.0114	¥910*0-	0.0752	0.0116	0.0030	0.0096	-9,0034	0.0155	0.0982	0.0088	-0.0103	0.0155	0.ņ115	0.0148	0.0128	0.0056	0.0221	0.0095	0.0150	0.0015	6210.0	0.0097	0.0118	0.0092	0400.0	0.0042	0.0242	0.0087	0.0076
JOHPONENT 258			ERR				ERR		ERR			583 8			ERR				EEB			ERR	ERR	EPR	EER	EPR	88	843	ER9	ESB	688 8	893	EFR	ERR	ERR	5RR	559	ERB	ERR
23 8			ERR	ERR	EAR	ERR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	ERR	ERR	5PP	ERR	ERR	ERR	ER8	ERR	5 P.P	ERR	ERR	ERR	ERR	ERR	EPR	ERR	EFF	ERR	ERA	483	ERR	ERP	883	E A B	EPP	ERR
822			ERR	ERR	589	5RR	EPS	ERR	EER	8¥3	883 883	ERR	EFR	ERR	ERR	ERA	ERR	ERR	ERA	ERR	ERA	ERR	EPR	ERR	ERR	E RR	ERR	EER	ERR	EFR	ERR	ERP	ERR	599	ERR	EFR	ERA	ERR	EPR
218			ERR	EBR	ERR	ERR	0.0222	-0-0005	ERR	EPR	ERR	ERR	EPR	ERR	-0-0149	0.0171	ERR	59.8	0.0027	0.0076	0.0205	-0.0210	-0.0008	0.0049	0.0012	-0.0084	0.0126	0.0016	-0.00.18	0.0003	0.0013	0.0012	0.0036	-0-0010	-0.0020	0.012	0.6095	-0 0014	0.000
80¥			14.079	16.158	16.145	16.292	16.296	16.374	16.371	I6.397	16.420	16.450	15.450	16.742	16.742	16.781	16.817	17.034	17.008	17.080	17.103	17.208	17,395	17,483	17.654	17.733	17-83B	17.989	18.153	18.317	18.527	19.61	18.848	19.193	19.442	19.537	19.596	19.672	022-61
398			18.710	18.737	18.740	18.835	18.947	18.976	19.048	19,045	19.048	19.061	19.081	19.337	19.403	19.419	19,442	19.649	19.659	147.47	19.741	19.793	19.954	20.078	20.213	29.338	20.393	20.561	20,689	20.892	21.105	21.243	21 394	21.716	21.991	22.047	22.680	22.188	22.290
feet) 388			13.979	13.996	14.012	1.127	14.183	14.199	14.242	14.268	14.262	14.334	14.327	14.511	14.537	34.616	14.616	11.816	14.855	14.898	11.885	[4.944	15.121	15.229	15.367	15.462	15.531	15.656	15.797	15 945	16.155	16.279	16.470	16.719	16.95B	17 034	17.624	17.159	17.237
BISPLACEMENT (in feet) 268 378 3			21.315	21 371	21.384	21.578	21.470	21.650	21.722	21.739	21.762	21.785	21.781	22.122	22.181	22.211	22.546	22.549	22.575	22.644	22.588	22.792	22.962	23.120	23, 356	23, 445	23.530	23.769	23.953	24.157	24.376	24 596	24, 813	25.170	25, 515	25.636	25.718	25.777	25.899
E BISPLACI 268			12, 956	12.956	12.969	13.928	13.038	13.107	13.156	13.215	13.199	13.179	13.215	13.416	13.432	13.455	13.452	13.648	13.668	13.70	13.685	13.740	13.878	13.967	14.108	14,160	14.229	14.347	14.528	H 577	14 739	14 847	15, 903	15.220	15.407	15.437	15.512	15.581	15.639
CUMBLATIVE 258			ERR	ERR	EPR	ERR	ERR	ERR	ERR	ERP	ERR	ERR	ERR	ERP	EPR	ERR	ERP	ERR	ERR	ERR	ERA	ERR	ERR	ERR	ERR	ERR	ERR	EFR	ERR	843	ERR	ERR	ERR	EPP	ERA	ERR	ERR	EPR	ERF
238			EAR	ERR	EPR	ERR	ERR	£88	ERA	ERR	E C C	ERP	599	883 1	693	ERR	588	66B	EFR	ERP	£F9	EPR	EFR	893	ERR	8 <u>9</u>	99.9	97. 17.	643	5.5	ERR	693	ERP	ERP	ERR	ERR	ERR	653	893
228			893	ERR	ERR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	ERR	ERR	EAA	ERR	EPR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	EPR	E99	ERR	EP.R	55 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	ERR	ERR	ERR	ERR	ERR	EPR	EPP	ERP	E 8.R	EPR
218			ERB	EPR	ERR	9.454	0.497	0.494	EKR EKR	EFR	ERR	EPR	EAR	9.500	0.418	9.464	ERR	0.458	0.464	0.504	0,543	9.467	9.458	0.487	0.500	0 22	9,46	0.491	0.138	0.441	0.458	0.471	0.520	0.497	9.454	0.484	0.513	0.470	0.490
		Bays	315.38	315.50	315.58	315, 89	315.96	316.25	316.36	316.42	316.48	316.53	316.55	317.27	317.50	317,60	317.64	318.17	318.27	318.94	318.52	318.67	317.17	312.42	319.88	329. 27	320.40	320.92	321.39	321,85	322.38	322.83	323, 40	324, 38	325.25	325,54	325.67	326.90	526.27
	ing Ing Bale	1.40	9.00																									-		• •					6.00	13, 60	15.00	24,00	6.30
128651	lero Prading Zero Frading Date	Date	=	ï		Ξ	Nev 11 79	2	<u></u>	21	<u>~</u>	2		::	5	13	<u>_</u>	-		_	Ξ	***	57	5		2	£ :	2	-		o:	61	6	2	2		г.,		

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TABLE R.S - ONE-DIMENSIONAL SLOPE DISPLACEMENTS AND VELOCITIES, SLOPE DISTANCE COMPONENT, PARE 3 0F 4.

Note: Slope Distance is the Direct Reading From the Electronic Bistance Measuring Instrument to the Target.

€ 0₽	Velocity Median	Bays	0.0014 326.37	0.0133 326.67				.,		0.0128 328.76															0.0087 335.17			0.7537 330.19 0.0050 372 44					0.0207 338.17			0.0109 339.13	0.0104 379.48	0.0033 335,84
348			0.0036	0.110	0100-0	9.0273	0,0082	0.0113	0.0118	0 0007	ERR	ERR	843	ERR	ERR	0.0278	593	EPP	0.0069	-0-050	0.0132	0.0067	0.0133	0,0083	0.0075	0.0055	0.015/	0.0000	2010 0	0.0075	0.0011	0.0108	0.0191	-0,0055	£9.00 Q	0.0115	0.0107	0.0005
ur) 388			0.0094	0.0078	0.0103	0.0150	0.0068	0.0116	0.0116	0 005l	0.9154	EPR	ERR	ERR	EPR	0.0220	0.0008	0.0178	0.0040	-0.0014	0.0162	0.0033	0.0078	0.0087	0.0141	-0.001	0.0176	0.0177	0.0081	0.0088	0.0076	0.0062	0.0054	0.0114	0.0067	0,0082	0110.0	0.0922
(feet/ho 378			0.0108	0.0140	0.0197	0.0059	0.0150	0.0046	0.0168	0.0140	0.0234	0.0122	0.0311	0.6061	0.0106	0.9240	0.0099	0.0159	0.0083	-0.0052	0.0269	0.0078	0.0125	0.0137	0.0099	0.0105	0, V13/	6714'0	0.0179	9.0079	0.0076	0.0137	0.0104	0.0068	0.0115	0.0115	0.0134	0.0093
COMPONENT VELOCITY (Feet/hour) 25B 26B 37B			0.0036	0.0114	0.0123	0.0013	0.0068	0.0130	0.0012	0.0120	0.0220	0.0051	-0.0261	0.0311	0.0019	0.0302	-0.0048	0.0150	0.0037	-0.0938	0.0107	0.0059	0.0086	0.0041	0.0124	0.0050	100.0	162410	0.0085	0.0071	0.0034	0.0075	0.0104	-0°0002	0.0052	0.0098	0.0095	-0.0115
OMPONENT 258			ERR	ERR	ERR	EPP	ERR	ERA	ERA	ERR	ERR	EPR	ER9	ERR	EER							ERH	EPA	ERR	ERR		1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1 1	693 1	568	1	ERR	ERG	ERR	ERR	ERR	ERR	ERR	ERR
τ 238			ERR	ERR	883	ERR	ERR	EPR	ERP	ERR	ERR	ERR	ERR	ERR	EPR	EPR	ERF	EPP	ERR	EPR	ERR	ERR	ERR	ERR	ERR.	EPR 202	195		E R R	E B	EPR	ERR	EPR	ERR	555	ERA	ERR	EAR
228			ERA	ERR	ERR	ERR	ERR	ERR	ERR	ERB	ERR	EPR	ERR	ERR	ERR	ERR	ERR	EFR	ERR	EPP	ERR	ERR	ERR	EPR	893 873	83 S		122 120	199	ERR	ERP	ERR	5 P. P	ERR	EPR	FR3	ERR	ERR
218			-0.0029	0.0026	-0.0057	0.0065	0.000	0.0014	-0.0034	0.0040	0.0088	-0.0024	-0.0162	-0.0015	0.0934	0.0149	- 0, 0058	0.0055	0.0010	-0.0071	0.0081	-0-0065	0.0008	0,0912	-0.0041	0.0009	0.0011	0-0120 -0.0104	0.0037	1000-0	-0.0034	0.0098	0.0075	9600.0-	0.6011	1100.0-	0.0039	-0°0044
+08 } {			19.777	119.911	29.033	20.115	20.144	20.266	20, 387	20.535					21.223			21.561	21.598			21.850				22.165 22.705	003''77	22.421	22.523	22.592	22.618	22.710	22.874	22.815	22.903	- 696.22	23.084	23.103
36£			22, 306	22.437	22.477	22.615	22, 634	22.743	22,890	23.002	ERR	ERR S	EAR	ERR	23.655	23.874	ERR	23.999	24.058	Z4.055	24.157	24.265	24.376	24,442	24.517	21.557	21 100	24, 780	21.872	24.978	24.957	25.042	25.193	25, 151	25.210	25.279	25.397	25.490
feet) 386			17.280	17.359	17.460	17.536	17.552	17.664	17.808	17.867	17.982	58B	ERR	ERR	18.530	18,794	18.710	18.838	18.927	18.917	19.042	19.094	19.176	19.245	19.357	19.347	DC - 11	19.577	19.449	19.714	19.780	19.829	19.872	19.954	20.013	20.052	20.183	29.197
UMMLATIVE DISPLACEMENT (in feel) 258 268 378 31			25.948	26.089	26.282	26.312	26.348	26.440	26.659	26.811	26.985	27.487	27.569	27.595	Z7.687	27.877	27.556	28.071	28, 153	28.116	28.323	28, 148	28.553	Z8.661	28.740	28.815	70 AUT	24.101	29.219	29.278	29.343	29.452	29.534	29.583	29.685	29.754	29.901	29.957
DISPLACE 26B			15, 647	15.761	15.983	15.876	15.893	16.017	16.970	14.208	16.372	16.582	16.513	16.647	16.664	16.903	16.876	17.095	17.041	17.015	17.097	17.192	17.264	17.297	17.395	17.45	11.4.11	17.582	17.658	17.710	17.740	17.793	17.881	17.877	17.923	17.982	18.087	18.019
UMULATIVE 258			ERR	ERR	ERR	ERR	ERR	888	ERR	ERR	588	566	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	52P	ERR	EPR	ER I	ERR 500	100		883	ERR	ERR	ERR	893	ERR	ERP	ERR	ERR	ERR
C 238			ERR	ERR	EFR	ERR	ERR	ERP	555	ER.R	ERR	ENR 1	ERP	EFF	EPH	ERR	EPR	EFR	683 1	643	ERR	686 6	ERR	88	EFR 222		80	ERR	ERP	ERR	508	ERR	ER9	EPF	Epg	ERR	999 199	ERR
228			ERB	EPR	ERR	ERR	ERR	ERP	EPR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERR	EPP	ERR	EPR	ERR	ERR	E88	88 F	EFR.			EPR	ERR	ERR	ERR	EPR	ERR	598	ERP	ERP	ERR	EPR
218			0.477	0.504	0.448	0.481	0.481	0.494	0.45	0.497	0.563	0 49 A	0.422	0.413	0.445	0.563	9.517	0.556	0.566	9.517	0.579	0.474	0.481	0.470	9.408	17 47 0	21X	0.451	0.484	0.487	0.458	91 (ŋ.523	0.454	9.464	0.458	0.500	0.474
****		Days	326.46	326.88	327.29	327.50	327.60	328.90	329.52	329.00	329.31	551.02	51.12	331.31	331.67	332.90	332, 33	332.63	333.04	333.33	333.65	334.32	334.67	335.00	527.55	176.00 176.00	334.25	336.63	337.00	337-31	337.67	338.00	338.33	338.63	229.00	339.25	339.71	339.96
	ig Date	11	00.11	21.00	7.09	12.00	14.30	24.99	12.3	24.90	57 L		۰. ۱	08.1	15.00	5 1. 00	8,00	15.00	1.00)	8.90	15.30	5°.'	16.00	2 93	10 P	(A.C)	4. 00	15.00	24,00	95.7	16.50	24.09	8.00	15.00	24.00	(i) 9	17, 00	23.00
135441	Tero Reading Tero feading Date	Date	Nov 22 79	22	23	Nov 23 79	22	23	2	2	10% 25 79 	35	21	121 22 19		27	Nov 28 79	00.1 F-4	2	6- 6-1	104 29 79	S :	8;	\$ 3	ŝł	5 Z	6	Der (12 79	20	3	8	6	Ū.	7	ġ.	Bec 05 79		

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TARLE R.S. - ONE-DIMENSIONAL SLORE DISPLACEMENTS AND VELOCITIES, SLOPE DISTANCE COMPONENT. PAGE 4 DF 4.

Note: Slope Distance is the Direct Reading from the Electronic Distance Measuring Instrument to the larget.

	Velocity Nedian	Days	340.11	340, 43	340.80	341,15	341°20	341.85	342.17	342.49	342.78	343,79	344.82	345.15	345.48	346,14	346.81	347.15	347,52	347.85	346.83		350,14	350.50	350.84	351,13	351.52	352, 04
80 4 [8 H.		0.0146	0.0195	843	ERP	0.0029	0.0172	0.0107	-0.0034	9600.0	0.0683	0.0166	-0°004	0.0088	1600.0	0.0148	0.0012	0.0218	0.0279	0.0622	0.0113	9.0110	0.0010	0.0077	0.0091	0.0051	0.0085
3 4 8			0.0141	0.0055	ESA	199 199	0.0029	0.0091	0.0115	- 9:00.5	7.0987	0.0076	0.0091	0.0012	0.0069	0.0084	0.0102	0.0025	0.0172	0.6244	0, 0008	0.0170	0,0053	0.0059	9.0033	0.0062	0.0048	0.0068
r} 388			0.0104	0.0086	643 643	ERR	0.0033	0.0111	0.0129	-0.0020	0.0109	0.0067	0.0137	0.0008	0.0088	0.0077	5010-0	0.0091	0.0098	0.0345	0.0013	0.0113	0.0097	0.0078	0.0049	0.0070	0.0086	0.9035
COMPONENT VELOCITY (feet/hour) 258 268 378			0.0118	0.0117	0.0109	0,0179	0.0068	0.0167	0.0140	0.0044	0.0141	0.0099	0.0133	0.0033	0.0125	0.0166	-0.0020	0.0041	ERR	ERR	ERR	0.0170	0.0128	0.0065	0.0082	0.0070	0.0169	0,0012
/ELOCITY 26B			0,0222	ERR	ERR	0.0207	0.0003	0.0101	0.0033	0.0019	9600 0	0.0060	0.0083	0.0033	0.0036	0.0078	0.0070	0.0021	67179.0	593	ERR	0.0104	0.0128	0.0003	0.0093	2900.0	0.0086	0.0028
DAPONENT 1 258			ERA	EF9	689	EPR	ERR	883	ERR	ERH	ERR	ERA	ERR	EKR	ERR	ERR	ERR	ERR	EPR	EPP	ERR	EPP	ERR	5PR	588	ERP.	559	EKR
23 B			ERR	ERR	ERA	669	ERR	EPR	ERR	£P.R	ERR	E P R	EPR	ERR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	ERE	ERP	E BB	ERR	EPR	EPR	ERR
22B			ERR	ESP	ESA	ESR	EPR	ERR	ERR	EPR	ERR	EPR	Efil	ERR	EKR	EPR	ERR	EPR	ERR	EPR	ERR	ERR	ERR	EPR	ERR	EPR	ERR	ERP
218			0.0033	-0.0008	-0.0027	0.0066	-0.0029	0.0056	-0,0055	-0.0029	0.0055	-0.0003	0.0021	-0.0041	-0,0024	0.0014	0.0031	-0-041	0.0130	-0.0160	0.0000	0.0019	0.0018	-9.0029	0.0016	0.0012	0.0039	-0.0038
] [408 2			23.205	23, 294	589	23.507	23. 536	23,648	23.740	23.717	23. 786	24.127	74.258	24.255	24.327	24.540	24.665	24.675	24.895	25.049	Z5. E H I	25.220	25.302	25.318	25.357	25.429	25, 485	25.603
39 8			25.49B	25.544	ERR	25.761	25, 790	25.849	25.951	25, 941	26.004	26.319	26 191	26.401	26.450	26.647	26.732	26.752	26.945	27.080	27.113	27.231	27.270	27.329	27.349	27.398	27.450	27.546
feet} 38B			20.269	20.341	699	20.535	20.567	20.640	20.754	20.741	20.820	21.096	21.204	21.210	21.283	21.463	21.552	Z1.624	21.722	21.912	21.968	22.047	22.119	22, 198	22.227	22.283	22.378	ZZ, 428
CUMULATIVE DISPLACEMENT (in feet) 258 268 378 3			30.039	30.137	30.242	39.367	30.436	30.544	30.669	30.698	30.800	31.210	31.355	31.341	31.443	31.834	31.817	31,850	ERR	ERR	32.352	32.470	32.565	32.631	32,680	32.736	32.923	32.939
015PLACE 268			18,173	ERR	18.268	18,412	IB. 415	18, 491	18.511	18.543	18.612	18.852	18.927	18,954	18.983	19.167	19.226	19.242	19.423	ERR	19.574	19.645	19.741	19.741	19.800	19.849	19.944	19.984
UHLATIVE 258			ERR	ERR	ERR	1893 1	ERR	ERP	ERR	EBB	ERR	ERR	ERR	ERR	ERR	ERR	ERR	ERP	ERR	EPP	EPR	ERR	ERR	ERR	ERR	ERR	ERR	ERR
C 23B			ERR	893	£68	893	EFR	EKP	E RR	ERR	EPR	Haga .	EPR	EPR	EFR	ERR	ERR	ERR	EPR	EPR	ERP	EPR	EPR	ERR	ERR	EEE	ERR	EPR
22B			5RR	ERR	ERR	EPR	ERR	ERR	ERR	ERP	ERR	ERR	EBR	ERR	ERR	ERR	ERR	EPR	ERR	ERR	ERR	ERR	ERK	ERR	ERR	ERR	EPR	ERR
[] 218			0.497	0.490	0.46	0.510	0,481	0.517	9.467	0.448	0.487	9 474	0 440	0.458	0.438	0.471	0 497	0 464	0,595	0.507	0.507	0.520	0.533	9.594	9.513	0.523	0.566	0.513
		Days	349.25	349.60	341.00	341.29	341.71	341.98	342.35	342.63	342,93	344.65	344.78	345.31	345.65	346.63	346.98	347.31	347.73	347.96	349.69	349.98	350.29	350.71	359.96	351.29	351.75	352.33
	ng Date	11.#8	6.09	H.30	24.00	7, (0)	17.00	23.39	8,30	15.00	00°22	15.30	23.34	7.30	15.30	15.00	23 TU	7.30	12 24	23.00	16.30	21, 30	7, 60	17,04	23.90	00.1	19.00	8, 90
TARGET	Zero Reading Zero Feading	Date	99	Bec 06 79	90	ē	÷	2	8⊖	8 0	80	9	ŝ		Ξ	12	2	Ľ	2	ï	5	ŝ	5	15	9	11	13	8
















DECDEEC

































ANGLES IN DEGREES



























APPENDIX C

PIEZOMETER DATA AND PLOTS

TAPLE C.1 - SEDURDWATER ELEVATIONS AND FIEZOMETER READINGS. PAGE 1 OF 2.

Mate: Fiecometer Nests B94, 007, and R08 Consist of 2, 3, and 3 Individual Standpipes per Hole, Respectively. Fiecometer 804A is Plugged and Exhibits Very Slow Response.

80 4 8 80 4 8
5575.70 5675.74 5643-74 5641.27 5641.22 5655.20 5796.09 5246.00 5475.00 5342.09 5194.09 5480.09 5446.00 5276.00 5573.00 5372.09 5224.99 5518.09
5570.68 5596.15 5567.31 5489.25
5514.42 5562.95 5488.86
3362.74 3334.75 5567 PA 5570 AD
5563.29 5559.37 5562.36 5486.27
5564.38 5552.01 5564.79 5484.73
5565.56 5550.41 5567.87 5484.79
5568.28 5549.49 5579.56 5484.27
5569.72 5548.90 5571.84
21'9/00 78'/100 94'1/00 0/ 103 1: 0733 13 0133
3005.32 33/2.33 3348.11 33/3.60 3404.73 3316.07 558.57 5577 AD 5547 AD 5547 AD 5577 41 5401 09 5564 79
5573.76 5548.50 5573.71
5573.56 5546.14 5573.25
5573.63 5545.91 5573.96
5573.30 5547.22 5572.27
5572.97 5549.03 5571.48
07 1/33
5570.91 5550.77 5568.27
5568.90 5548.93 5566.17
5567.40 5546.67 5564.13
5566.02 5544.14 5562.59
5564.77 5542.30 5560.95
3046.63 3263.19 3341.78 3339.80 3477.64 3496.36 5586 71 5561 79 5540 84 5557 27 5478 40 5406 72
5557.78 5543.29 5556.26
5558.14 5543.68 5554.03
86.39 5556.50 5541.52 5552.39 5476.66
ERR 5542.11 5551.53 5476.29
ERR EPR 5542.11 5550.55 5475.93 5500.46
ERR 5539.22 5546.45
EPR 5538.46 5545.40
ERR 5536.37 5543.50
ERR ERR 5533.84 5540.48 5146.60

TARLE C.1 - GROUNDHATER ELEVATIONS AND PIEZONFIER READINGS. PAGE 2 OF 2.

Note: Piezometer Nests 804, 807, and 808 Consist of 2, 3, and 3 Individual Standpipes per Hole, Respectively. Fictometer 8044 is Plugged and Ethibits Very Slow Response.

PIEZOMETER BROPLINE READINGS (in ≪etres) B04B 807A 8078 807C 808A 8 B04A -----} [-----3808 8088 B04A B04B 807A B07A B07A B08A GROUNDWATER ELEVATIONS (in feet) -----] FIEZOMETER

3808C

8088

		16.23	17.55	17.83	71.56	ERR	19.53	56.48	65,67
		41.87	12.52	12,00	43.99	ERR	44,82	38.65	11.31
		41.31	41.27	41.15	11.31	ERR	11.34	39.74	11:31
		60.12	60,24	59.05	59.32	ERR	57.08	64.69	58,13
		32.48	32.37	33.49	36.44	EPR	38.13	45.02	40.56
		33.43	34.70	34.75	38.38	ERR	39.13	11.32	39.81
		ERR	ERR	ERR	EPR	50.89	ERR	ERR	54.31
		ERA	ERR	ERR	ERR	ERR	ERR	ERR	ERR
5636.83 5190.00 5220.00		5386.73	5382.40	5381.49	5402.06	ERR	5430.24	5451.53	5421.38
5635.60 5339.00 5369.00		5498.23	5476.10	5497.81	5491.28	ERA	5488.55	5508.89	5590.07
5635.20 5488.00 5518.00		5499.47	5499.80	5500.20	5499.67	ERR	5439.57	5504.82	5499.47
5641.22 5194.00 5224.00		5443,98	5443.58	5447.49	5446.60	EPR	5453,95	5430.95	5450.51
5641.69 5342.90 5372.00		5535.13	5535.49	5531.16	5522.14	ERR.	5516.59	5473.99	5508.62
5639.94 5493.00 5523.09		5530.26	5526.10	5525.93	5514.07	ERR	5511.56	5594.38	5509.33
5695,74 5246,00 5296,00		ERR	883 1	ERR	ERR	5528.78	ERR	ERP	5517.56
5695,90 5396,00 5446,00		EPA	ERR	ERR	E88	ERR	EPR	ERR	ERR
aase Cop	SÁPQ	331.50	334.50	339.50	362.59	369.50	381.59	387.50	414.50
ation vation, (T I R C	12.69	12,00	17, AU	12.09	12.00	(129)	12.00	12.99
Batum Elevation Screen Elevation, base top	Date	May 27 75	80.4 50 79	Pec 05 79	Dec 28 79	Jan 04 30	÷	Jan 22 80	Feb 18 Bi






APPENDIX D

LOG OF COREHOLE E21-R50

<u>,</u>		PIEZOMETER						
		OTHER TEST RESULTS						FIGURE D.1- E21-R50
Ī		DOUBLE SHEAR						
l	GINEEKING	FRACTURE FREQUENCY PER FOOT						
	E Z C	V POOR DESIGNATION V POOR DESIGNATION 25 50 FAIR TON 75 6000						
		SS3N08AH				жд Ж	R1 R1	<u>T</u>
		WATER LOSS ZZZ				121	///	////
		BROKEN CORE						
ſ		GRAPHIC LOG						
	61C L06	DESCRIPTION AND REMARKS	Casing to 20' HQ core to begin with then change to NQ at 82'	Hole Station E21-R50 -60° to the N 30 E	20-28.2 Very soft Carbonaceous sandy clay 28.2-96 oxidized silty sandstone		1" of clay (carbonaceous) at 44.9	Fault Zone 57'-62' Broken core, iron stained gouge
	0			20	23 28 28	32 37	43	52 57
	GEOL				2 2 2		Fe 75 Fe 83 Fe 83	ອ ັດ ສຸ ກັ <u>ເ</u>
	GE				ىت قە االا			
				· · · · · · · · · · · · · · · · · · ·				
		PERCENT CORE LOSS 25 50 75						
		Ф ОЕРТН	10	20				

Γ	PIEZOMETER PIEZOMETER						
	OTHER TEST RESULTS						FIGURE D.1 E21-R50
) LOG	DOUBLE SHEAR						
NGINEERING	FRACTURE FREQUENCY PER FOOT 1 2 3						
ENG	ROCK QUALITY DESIGNATION V POOR V V POOR V V POOR V V V V V V V V V V V V V V V V V V V						
	CEMENT CO	<u>R1</u>	R3 R3	R3	R3	R3	
		VA A					
	ек⊾рніс гое						•
c L06	DESCRIPTION AND REMARKS			Now drilling with NQ at 82' 96-138 slightly silty sandstone	Continuing good solid rock. Iron oxide stain- ing down to 96' except on some joints as noted	Sandstone has small (1/16 carbonaceous shale layers	
0610		62 67	72	82 87.5	92 97	102	112 117
GEOL		6	70 30 78	5 2 S	60 64 83 60 64	80 90 It	24 76 75 75
ш Ю	ТҮРЕ ИКСЦИАТІОИ С С С С С С С С С С С С С С С С С С С	<u> </u>	3 2 <u>8</u> 2	2 2 2	1 Fe		of the state
	түре б	بة × ×		à a ă ă >•			
	PERCENT CORE LOSS 25 50 75						
	нтаза 🕱		80	06	100	110	120

Γ		IE ZOMETER NOTALATION	ਹ ਹ														
		OTHER TEST RESULTS			·····	r		T	1	1 1	1		T			FIGURE D.1	2 of 4
6 LOG	Ľ	DOUBLE Shear															
NGINEFRING		FRACTURE FREQUENCY PER FOOT I 2 3						1									
ENG	,	СК СК СК СК СК СК СК ССК ССК ССС ССС СС	·														
		SZENDAAH	R2		TS IS	R2		R2		R3	R3		•••		+		
		WATER LOSS															

		6RAPHIC LOG	••••••	. ·					•	• • •				·			
61C L06		DESCRIPTION AND REMARKS	Core tube did not lock ground core	133 3" soft carbonaceous	ciay 135 1" " " clay 137 3" " " " "		138-141 Silty sandstone with some soft clay	layers 141 Slightly silty sand- stone							Iron stained joint - fault? making water at	20 gal/min	
r 0 G I			122 127	132	137		142	147	152	4) {	157	162	167	• •	172	177	
GEO L	FRACTURES		80 20	CIAN 70		+dey 75 85		86	576	17	17	75	75	75	75 35	151	
υ		TYPE	1111 × × × × × × × • 1111			}•+ +d		2 2 2 2 2 3 3 3 3 4 3 3 4 3 4 3 4 3 4 3	10	55 55 111111	63	ر آ	ు 	- e I C	- F1-11?	â]
		PERCENT CORE LOSS 25 50 75					<u></u>					Ī	<u>I</u>		<u> </u>		
		т 13 оертн	130		1 1 1	140	1-1			1 1	160	11	1	170	<u> </u>	1	٥ م

Γ	PIEZOMETER NOTA JATZNI	Ţ				
	OTHER TEST RESULTS					FIGURE D.1 E21-R50 4 of 4
POG	DOUBLE SHEAR					
NGINEERING	FRACTURE FREQUENCY .PER FOOT					
ENG	ROCK QUALITY DESIGNATION SPOOR RATION S5000 CFAIR S5000 CFAIR S9EXCEL	1				
	RERNESS	R3 R3 R3	R3 R3 R3			
	WATER LOSS (ZZZ)					
	CRAPHIC LOG	·		•		
	X S N		ground			
IC LOG	DESCRIPTION AND REMARKS		Tube didn't catch core	TD 201'		
0 G I C L	DESCRIP. AND REMAR	182 187	192 197 Tube didn't c	201 TD 201'		
0 G I C L			192 197 Tube core	Ê		
			rrg day 192 vrg. 197 Tube core	201 TD		
0 G I C L			192 197 Tube core	201 TD		

APPENDIX E

SLOPE STABILITY ANALYSES

TABLE E.1 - SUMMARY OF STABILITY ANALYSIS TRIALS

Figure	Trial	Failure	Phreatic	Fric	tion Ar	gles	Factor	Comments
Number	Number	Surface	Surface	ϕ_1	<u>\$2</u>	<u>Ø3</u>	of Safety	
			100 000	050	~ ~ 1			
E.1	AMT2	Bilinear	180-200	25°	35'	38'	0.985	No buttress
E.2	AMT3	\$1	It	11	11	18	1.433	With buttress
E.3	AMT4	91	140-160	13	L‡	H	1.145	No buttress, Dewatered 40 ft.
E.4	AMT5	11	100-120	"	11	11	1.259	No buttress, Dewatered 80 ft.
E.5	AMT'6	11	6080	18	01	11	1.308	No buttress, Completely Dewatered
E.6	AMT43	19	180-200	H	F1	**	1.206	Half width buttress
E.7	AMT44	19	11	16	1.	H	1.269	Half height buttress
E.8	AMT12	Curve 1	u	••	11	18	1.121	No buttress
E.9	AMT13	81	#1	11	H	It	1.503	With buttress
E.10	AMT15	13	30-30	н	14	"	1.411	No buttress, Completely dewatered
E.11	AMT14	Curve 2	180-200	11	11		1.137	No buttress
E.12	AMT22	Circles	"	13	F1	11	1.130	No buttress Circle through toe
	AMT32AC	Curve l	180-200	25°	31 . 3°	38°	0.986	Back calculate ϕ_2 to match AMT2
-	AMT33AC	It	11	*1	м	18	1.368	With buttress
	AMT22AC	Circles	11	11	11	¥1	0.941	No buttress
-	AMT12C	Curve l	200-200	25°	35°	38°	1.027	Higher groundwater profile
-	AMT12CS	19	*1	"	B1	11	1.024	25 ft. slices (vs. 50 ft. standard)
-	AMT7E	Bilinear	30-30	24°	24°	38°	0.950	Back analysis, completely dewate red

TABLE E.1 - SUMMARY OF STABILITY ANALYSIS TRIALS (Continued)

Figure Number	Trial Number	Failure Surface	Phreatic Surface	Fric	tion A $\frac{\varphi_2}{2}$	ngles $\frac{\phi_3}{2}$	Factor of Safety	Comments
2 	AMT52	Bilinear	180-200	32.1	32.1	38°	0.984	No buttress
••••	AMT53	"	11	#1	31	¥1.	1.550	With buttress
-	AMT54	11	140-160	11	11	**	1.154	No buttress, Dewatered 40 ft.
-	AMT55	11	100-120	ă I	18	11	1.282	No buttress, Dewatered 80 ft.
	AMT56	u	30-30	81	**	13	1.343	No buttress, Completely Dewate red
	AMT62	34	" C.	$0 = c_2$	0 = 3900	38 lb./ft	0.984 .2	No buttress, Undrained analysis
-	AMT63	н	п	ŧŀ	"	н	1.053	With buttress, Undrained analysis
_	AMT72	Curve l	180-200	32.1	32.1	38°	1.037	No buttress
-	AMT73	0	**	11	"	*1	1.517	With buttress
-	AMT75	11	30-30	11	и	11	1.333	No buttress Completely dewatered
-	AMT74	Curve 2	180-200	11	"	13	1.049	No buttress
	AMT82AC	Curve 1	18	30.8	30.8	18	0.986	Back calculate ϕ_2 to match AMT52
	AMT83AC	11	18	11	н	11	1.449	With buttress
	AMT92	11	30-30 c ₁	0 = c ₂ =	0 39001	38 b./ft.	1.078 2	No buttress Undrained Analysis
	AMT93	11	11	11	Ħ	Ħ	1.142	With buttress Undrained Analysis

TABLE E.1 - SUMMARY OF STABILITY ANALYSIS (continued)

NOTES:

- Failure Surface: "Curve 1" and "Curve 2" are identical and parallel to the measured slope displacements, except that Curve 2 has no upward inclination at the toe of the failure.
- 2. Phreatic Surface: "180-200" refers to the elevation of the upper portion of the phreatic surface; "180" equates to 5480 feet (1670 metres) elevation at the "crest" of the piezometric surface near the slope; "200" equates to 5500 feet (1676 meters) the measured piezometric elevation is piezometer nest B08 distant from the slope.
- 3. Friction Angles: $|\phi_1|$ applies to disturbed material at the toe of the slope; $|\phi_2|$ applies to strata above the toe, parallel to bedding; $|\phi_3|$ applies to the rock fill buttress.

PROJECT : CRC PIT S1-B-2 ANALYSIS DESCRIPTION : AMT2

FIGURE E.1

JANDU ITERATIVE ANALYSIS

CORRECTION FRCTOR: 1.039 Fos convergence:

.989 .986

FINAL FOS = .985 RESISTING FORCE- 2957277 DRIVING FORCE- 3117992 <u>PROJECT</u> :CRC PIT 51-8-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-8-2 MATERIAL PROPERTIES

NUMBER OF LAYERS : 3 NATER DENSITY : 62.4 (166/613)

		L PROPERTIES (16f/ft3) (CONESION :	8,8 (16f/ft2)	PHI : 23.0 (deg)
LRYER . Densit	2 MATERIA 4 : 130.0	L PROPERTIES (1667/643) (CONESSION :	0.0 (16f/ft2)	PHI : 35.0 (deg)
LAYER D Densit	3 MATERIA Y : 125.0	L PROPERTIES ({ bf/fig> (COHESION :	0.8 (16f/f12)	PHI : 30.0 (deg)
PROJECT	CRC PIT 5	1-8-2			
LAYER GE	OMETRY				
LAYER .	1 SURFACE	(FT)			
		. 8 365.8 394	8.8 (808.8		
Z :	6.9 56	1.8 58.8 73	2.0 72.0		
LRYER #	2 SURFACE	(FT)			
Xt	a.e e	.0 365.0 72	8.8 1888.8		
Z:	0.0 50	1.1 50.1 38	5.6 305.0		
LAYER .	3 SURFACE	E (F1)			
x	0.0 0	0.0 365.0 72 1.2 50.2 30	8.8 1996.9		
2:	10.00 Di	1.2 38.2 38	2,1 3492/I		
PHREATIC	SURFACE ((FT)			
X:	0.6 36	5,8 598.8 188	0.0		
Z :	30.0 45	9.9 188.0 28	8.8		
	SURFACE (
		LDTH (FT): 50			
		5.0 590.0 68 9.3 72.0 38			
4.	- C. B. C.				

PROJECT :CRC PIT 51-8-2 ANALYSIS DESCRIPTION : AMT3

FIGURE E.2

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.040 FOS CONVERGENCE: 1.338 1.416 1.430 1.432 FINAL FOS = 1.433 RESISTING FORCE= 4605747 DRIVING FORCE= 3342560 PROJECT :CRC PIT 51-B-2

DATA LIST FOR FILE :

SLOPE GEOMETRY



```
PROJECT :CRC PIT 51-8-2
 MATERIAL PROPERTIES
 NUMBER OF LAYERS : 3
Water Demsity : 62.4 (166/f13)
LAYER 6 I MATERIAL PROPERTIES
Density : 120.0 (10//f13) comesion :
                                                                       0.0 (15f/ft2) PHI : 25.0 (deg)
LAYER # 2 MATERIAL PROPERTIES
DENSITY : 130.0 (165/113) COMESION :
                                                                       0.0 (156/612) PHI 1 35.0 (deg)
LAYER 6 3 MATERIAL PROPERTIES
Density : 125.8 (166/413) Cohesion ;
                                                                       0.0 (16f/ft2) PHI : 30.0 (deg)
PROJECT :CRC PIT 51-8-2
LAYER GEOMETRY
LAYER # I SURFACE (FT)
X: 0.0 0.8 365.0 390.0 1000.0
Z: 0.0 50.0 50.0 72.0 72.0
LAYER # 2 SURFACE (FT)
X: 8.0 0.0 365.0 720.0 1000.0
Z: 0.0 50.1 50.1 305.0 305.0
LAYER • 3 SURFACE (FT)
X: 0.0 0.9 235.0 497.0 612.0 720.0 1000.0
Z: 0.0 50.2 50.2 230.0 230.0 305.1 305.1
PHREATIC SURFACE (FT)
X: 0.8 365.8 590.8 1080.8
2: 30.8 49.9 180.8 200.0
FRILURE SURFACE (FT)
MAXIMUM SLICE HIDTH (FT): 50.0
X: 255.0 365.0 590.0 880.0 800.1
Z: 50.3 50.3 72.0 300.0 305.2
```

PROJECT :CRC PIT 51-B-2 RNALVSIS DESCRIPTION :AMT4

FIGURE E.3

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.039 FOS CONVERGENCE: 1.103 1.134 1.142 1.144 FINAL FOS = 1.145 RESISTING FORCE= 3434665 DRIVING FORCE= 3117992 <u>PRUJECT</u> :CRC PIT 51-B-2

DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-8-2 MATERIAL PROPERTIES

NUMBER OF LAYERS : 3 HATER DEHSITY : 62.4 (lbf/ft3) LAYER 0 1 HATERIAL PROPERTIES DEHSITY : 120.0 (lbf/ft3) COHESION : 0.0 (lbf/ft2) PHI : 25.0 (deg) LAYER 0 2 HATERIAL PROPERTIES DENSITY : 130.0 (lbf/ft3) COHESION : 0.0 (lbf/ft2) PHI : 35.0 (deg) LAYER 0 3 MATERIAL PROPERTIES DEHSITY : 123.0 (lbf/ft3) COHESION : 0.0 (lbf/ft2) PHI : 30.0 (deg) <u>PROJECT</u> :CRC PIT 51-B-2 LAYER 0 1 SURFACE (FT) X: 0.0 0.0 365.0 390.0 1000.0 Z: 0.0 50.0 50.0 72.0 72.0 LAYER 0 2 SURFACE (FT) X: 0.0 0.0 365.0 720.0 1000.0 Z: 0.0 50.1 30.1 305.0 305.0 LAYER 0 3 SURFACE (FT) X: 0.0 0.0 365.0 720.0 1000.0 Z: 0.0 50.2 50.2 305.1 305.1 PHREATIC SURFACE (FT) X: 0.0 365.0 590.0 1000.0 Z: 0.0 365.0 590.0 000.0 ANAINUM SLICE WIDTH (FT): 50.0 X: 0.0 365.0 590.0 000.0 305.2 CAUER 0 25.0 365.0 590.0 000.0 305.2 PROJECT : CRC PIT 51-B-2 ANALYSIS DESCRIPTION : AMTS

FIGURE E.4

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.039 FOS CONVERGENCE: 1.186 1.241 1.255 1.250 Final Fos = 1.259 Resisting Force= 3778118 Driving Force= 3117992 <u>PROJECT</u> :CRC PIT 51-B-2 DRIA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-8-2 MATERIAL PROPERTIES

NUMBER OF LAYERS 1 3 Water Density ; 52.4 (156/f13)

LAYER . Densiti	1 MATE 7 1 12	₽1AL ₽# 9.0 (}}	ROPERTI of/f13)	ES COHE	510N ;	0.8 (16f/ft2)	PHI : 25.0 (deg)
LAYER . Density	2 MATE 7 : 13	RIAL PI 0.0 (1)	ROPERII 66/ft3)	ES Cohe	SICH :	8.9 (16f/f12)	PHI : 35.0 (deg)
LAYER # Densit	3 MATE Y : 12	RIAL PI 5.0 (1)	ROPERTI 57/f13)	ES Come	SION :	0.0 (16F/f12)	PHI 1 38.6 (deg)
PROJECT	CRC PI:	T 51-B	-2				
LAYER GE	OMETRY						
LAYER .							
X:	0.6	6.9	365.8	398.0 72.0	22.6		
<i>x</i> :	0.0	30.0	2010				
LAYER #	2 CHAR		T 1				
	2 QUKF	8.0	365.0	728.8	1666.8		
2:	0.0	50.1	50.1	305.0	385.8		
LAYER .	3 SURF	ACE (F	тэ				
x:	8.9	0.0	365.8	728.0 305.1	1888.8		
Z:	0,0	59.2	50.2	305.1	305.1		
PHREATIC							
				1000.8			
Z:	36.9	49.9	100.8	128.8			
#64.UF=							
FALLURE		E (F3) E WIDTH		50 0			
				866.6	688.1		
Z:	59.3	50.3	72.8	366.6	365.2		

.

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.039 FOS CONVERGENCE: 1.223 1.260 1.304 1.307 FINAL FOS = 1.300 RESISTING FORCE= 3925620 DRIVING FORCE= 3117992 <u>PROJECT</u> :CRC PIT 51-B-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



<u>PROJECT</u> :CRC PIT 51-B-2 Material properties

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.040 FOS CONVERGENCE: 1.154 1.194 1.203 1.205 FINAL FOS = 1,286 RESISTING FORCE= 3782268 DRIVING FORCE= 3261116 PROJECT :CRC PIT 51-8-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



8.8 (15f/f12) PHI : 35.8 (deg)

PROJECT : CRC PIT 51-B-2 MATERIAL PROPERTIES NUMBER OF LAYERS : 3 WATER DENSITY : 62.4 (167/ft3) LAYER # ; MATERIAL PROPERTIES Density : 120.0 (167/513) Comesign ; 8.8 (16F/ft2) PHI : 25.8 (deg) LAYER . 2 MATERIAL PROPERTIES Density : 130.0 (16//f13) comesion : LAYER # 3 NATERIAL PROPERTIES Density : 125.0 (16//f13) Comesion : 0.8 (15f/f12) PHI : 38.8 (deg) PROJECT :CRC PIT 51-B-2 LAYER GEOMETRY LAYER # 1 SURFACE (FT) X: 0.6 0.0 365.0 390.0 1000.0 Z: 0.0 50.0 50.0 72.0 72.0

LAYER 0 2 SURFACE (FT) X: 0.0 0.0 365.0 720.0 1000.0 Z: 0.0 50.1 50.1 305.0 305.0 LAYER # 3 SURFACE (FT) X: 0.0 0.0 310.0 555.0 612.0 720.0 1000.0 Z: 0.0 50.2 50.2 230.0 230.0 305.1 305.1 PHREATIC SURFACE (FT) X: 0.0 365.0 390.0 1000.0 Z: 30.0 49.9 100.0 200.0

FRILURE SURFACE (FT) Haxinum Slice Hidth (FT): 30.0 X: 255.0 365.0 596.0 888.0 808.1 Z: 56.3 56.3 72.0 300.0 305.2

PROJECT :CRC PIT 51-B-2 ANALYSIS DESCRIPTION :AMT44

FIGURE E.7

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.039 FOS CONVERGENCE: 1.284 1.256 1.267 1.269 FINAL FOS = 1.269 RESISTING FORCE- 3092905 DRIVING FORCE- 3103623 <u>PROJECT</u> :CRC PIT 51-B-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-B-2 MATERIAL PROPERTIES

HUMBER OF LAYERS : 3 HATER DEHSITY : 62.4 (167/f13) LAYER 4 1 MATERIAL PROPERTIES DENSITY : 120.8 (167/f13) COHESION : 0.8 (167/f12) PHI : 23.6 (deg) LAYER 4 2 MATERIAL PROPERTIES DENSITY : 130.8 (167/f13) COHESION : 0.8 (167/f12) PHI : 35.0 (deg) LAYER 4 3 MATERIAL PROPERTIES DEHSITY : 125.9 (167/f13) COHESION : 0.8 (167/f12) PHI : 30.8 (deg) PROJECT : (CCC PIT 51-B-2 LAYER 4 1 SURFACE (FT) X1 0.8 0.8 365.8 390.0 1000.8 Z1 0.8 50.9 50.7 72.0 72.0 LAYER 4 2 SURFACE (FT) X1 0.8 0.8 365.6 720.0 72.0 LAYER 4 3 SURFACE (FT) X1 0.8 0.8 355.6 720.8 1000.8 Z1 0.9 50.1 50.1 305.0 305.0 LAYER 4 3 SURFACE (FT) X1 0.8 0.8 255.8 376.8 492.8 720.8 1000.8 Z1 0.9 50.1 50.1 305.0 305.0 LAYER 4 3 SURFACE (FT) X1 0.8 0.8 255.0 376.8 492.8 720.8 1000.8 Z1 0.9 50.2 50.2 140.8 140.8 305.1 305.1 PHREATIC SURFACE (FT) X1 0.8 365.0 590.8 1000.8 Z1 30.8 49.9 150.8 200.8 FAILURE SURFACE (FT) MAXINUM SLICE HIDTH (FT): 50.8 X1 255.6 355.0 590.8 000.8 000.1 Z1 50.3 50.3 72.0 305.2

LAYER & 2 MATERIAL PROPERTIES DENSITY : 130.0 (167/ft3) COMESION : 0.0 (167/ft2) PHI : 35.0 (deq) LAYER & 3 MATERIAL PROPERTIES DENSITY : 125.0 (167/ft3) COMESION : 0.0 (167/ft2) PHI : 30.0 (deq) <u>PROJECT</u> :CRC PIT 51-B-2 LAYER & 1 SURFACE (FT) X: 0.0 50.0 50.0 72.0 72.0 LAYER & 2 SURFACE (FT) X: 0.0 50.1 50.1 305.0 1000.0 Z: 0.0 50.1 50.1 305.0 303.0 LAYER & 3 SURFACE (FT) X: 0.8 50.1 50.1 305.0 303.0 LAYER & 3 SURFACE (FT) X: 0.8 50.2 50.2 305.1 305.1 PHREATIC SURFACE (FT) X: 0.6 50.2 50.2 305.1 305.1 PHREATIC SURFACE (FT) X: 0.6 50.6 590.6 1000.0 Z: 30.0 49.9 100.0 200.0 FAILURE SURFACE (FT) MAXIMUM SLICE WIDTH (FT): 50.0 X: 25.0 365.0 393.0 501.0 610.0 800.0 800.1 Z: 50.3 50.3 440 73.0 120.0 202.0 305.2

8.8 (15f/f12) PHI : 25.8 (deg)

PROJECT :CRC PIT 51-8-2 MATERIAL PROPERTIES

NUMBER OF LAYERS : 3 HATER DENSITY : 52.4 (16//f13)

LAYER # 1 MATERIAL PROPERTIES DENSITY : 120.0 (16//f13) COMESION :



SLOPE GEOMETRY

DATA LIST FOR FILE :

CORRECTION FACTOR: 1.831 FOS CONVERGENCE: 1.889 1.113 1.121 FINAL FOS = 1.121 RESISTING FORCE= 2828387 DRIVING FORCE= 2593293 PF0JECT :CRC PIT 51-B-2

PROJECT :CRC PIT 51-B-2 BHALYSIS DESCRIPTION :BMT12

JANBU ITERATIVE ANALYSIS

PROJECT :CRC PIT 51-8-2 ANALYSIS DESCRIPTION :AMTI3

FIGURE E.9

JANBU ITERATIVE ANALYSIS

CORRECTION FACTOR: 1.03; Fos convergence:

> 1.399 1.406 1.500 1.503

FINAL FOS = 1.303 RESISTING FORCE= 4511174 DRIVING FORCE= 3093040 <u>PROJECT</u> :CRC PIT 51-B-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-B-2 MATERIAL PROPERTIES

HUMBER OF LAYERS : 3 WATER DENSITY : 62.4 (166/613)

		IAL PROPERT .8 (15f/f13		0.8 (15f/fi2)	PHI : 23.8 (deg)
		IAL PROPERT .0 (15f/ft3		8.8 (15f/fi2)	PH1 : 35.0 (deg)
		IAL PROPERT		0.8 (15f/ft2)	PHI : 38.0 (deg)
PROJECT	;CRC PIT	51-8-2			
LAYER G	LUNEIKY				
18458 6	I SURFA	CE (ET)			
			398.0 1000.0		
2:			72.0 72.0		
LHYER 10 X:	2 SURFR	CE (FT)	728,0 1800.0		
ž:	8.8	56.1 50.1	365.0 305.0		
LAYER #	3 SURFA	CE (FT)			
x:			497.8 612.8	720.0 1000.0	
Ζ:	0.8	50.2 50.2	238.0 230.0	385.1 305.1	
0405071	C SURFACE	157)			
		63.9 399.8	1999.9		
		49.9 100.0			
FAILURE	SURFACE	(FT)			
		MIDTH (FT):	50.0		
			501.0 610.0		
Z:	50.3	58,3 44.8	73.0 120.0	202.0 305.2	

PROJECT (CRC PIT 51-8-2 ANALYSIS DESCRIPTION (AMT15)

FIGURE E.10

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JANBU ITERATIVE ANALYSIS
```

CORRECTION FACTOR: 1.031 Fos convergence;



FINAL FOS = 1,411 RESISTING FORCE- 3330634 DRIVING FORCE- 2593293 <u>PROJECT</u> :CRC PIT 51-8-2 DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-B-2 MATERIAL PROPERTIES

```
HUMBER OF LAYERS : 3

HATER DEHSITY : 62.4 (167/f13)

LAYER 0 1 HATERIAL PROPERTIES

DENSITY : 120.8 (167/f13) COMESION : 0.0 (167/f12) PHI : 25.0 (deg)

LAYER 0 2 HATERIAL PROPERTIES

DEHSITY : 130.0 (167/f13) COMESION : 0.0 (167/f12) PHI : 35.0 (deg)

LAYER 0 3 HATERIAL PROPERTIES

DEHSITY : 125.0 (167/f13) COMESION : 0.0 (167/f12) PHI : 30.0 (deg)

PPOJECI :(CRC FIT 51-B-2

LAYER 0 1 SURFACE (FT)

X: 0.0 0.0 365.0 398.0 1000.0

Z: 0.0 50.0 50.0 72.0 72.0

LAYER 4 2 SURFACE (FT)

X: 0.0 50.1 305.0 1000.0

Z: 0.0 50.1 305.0 1000.0

Z: 0.0 50.2 50.2 305.1 305.1

PHREATIC SURFACE (FT)

X: -.7 1006.7

Z: 30.6 30.6

FAILURE SURFACE (FT)

MAXIMUM SLICE MIDTH (FT): 50.0

X: 250.3 355.0 301.0 618.0 000.8 600.1

Z: 50.3 305.0 301.0 618.0 202.0 300.2
```

PROJECT :CRC PIT 51-8-2 ANALYSIS DESCRIPTION :ANTI4

FIGURE E.11



CORRECTION FRCTOR: 1.031 FOS CONVERGENCE: 1.101 1.129 1.135 1.137 FINAL FOS = 1.137 RESISTING FORCE= 2017939 DRIVING FORCE= 2353433 PR0JECT : CRC PIT 51-B-2

DATA LIST FOR FILE :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-B-2 MATERIAL PROPERTIES

NUMBER OF LAYERS : 3 Hater Density : 62.4 (166/ft3)

```
LAYER # 1 MATERIAL PROPERTIES

DENSITY : 128.0 (1bf/f13) COMESION : 0.0 (1bf/f12) PHI : 25.0 (deg)

LAYER # 2 MATERIAL PROPERTIES

DENSITY : 138.0 (1bf/f13) COMESION : 0.0 (1bf/f12) PHI : 35.0 (deg)

LAYER # 3 MATERIAL PROPERTIES

DENSITY : 125.0 (1bf/f13) COMESION : 0.0 (1bf/f12) PHI : 35.0 (deg)

PROJECT :CRC PIT 51-8-2

LAYER # 1 SURFACE (FT)

X: 0.8 0.0 365.0 390.0 1000.0

Z: 0.0 50.0 50.0 72.0 72.0

LAYER # 2 SURFACE (FT)

X: 0.0 0.0 365.0 720.0 1000.0

Z: 0.0 50.2 50.2 305.1 305.0

LAYER # 3 SURFACE (FT)

X: 0.0 365.0 590.0 1000.0

Z: 0.0 50.2 50.2 305.1 305.1

PHREATIC SURFACE (FT)

X: 0.0 365.0 590.0 1000.0

Z: 30.0 49.9 100.0 200.0

FAILURE SURFACE (FT)

MAXIMUM SLICE WIDTH (FT): 30.0

X: 73.0 120.0 500.0 800.1

Z: 50.3 59.3 50.3 73.0 120.0 202.0 305.2
```

 PROJECT
 :CRC PIT 51-8-2

 ANALYSIS
 DESCRIPTION
 :AMT22
 FIGURE E.12

 JAHRU SIS
 DESCRIPTION
 :AMT22
 FIGURE E.12

 JAHRU SIS
 DESCRIPTION
 :AMT23
 FIGURE E.12

 JAHRU SIS
 1.023
 1.017
 119641
 11074

 RABIUS
 420
 1.025
 1.021
 100521
 95727

 RABIUS
 1.116
 1.031
 20371
 100522
 21172

 RABIUS
 1.051
 1.031
 1.032
 224471
 103722

 RABIUS
 2.061
 1.061
 1.033
 224672
 246172

 RAB



PROJECT :CRC PIT 51-8-2 Date list for file :

SLOPE GEOMETRY



PROJECT :CRC PIT 51-B-2 MATERIAL PROPERTIES

HUMBER OF LAYERS : 3 WATER DEHSITY : 62.4 (155/f13)

LAYER & L MATERIAL PROPERTIES DENSITY : 128.8 (16//13) CONESION : 0.8 (16//12) PH1 : 25.8 (deg) LAYER & 2 MATERIAL PROPERTIES DENSITY : 128.8 (16//13) CONESION : 0.8 (16//12) PH1 : 35.8 (deg) LAYER & 3 MATERIAL PROPERTIES DENSITY : 128.8 (16//13) CONESION : 0.8 (16//12) PH1 : 38.8 (deg) <u>PROJECT</u> :CRC PIT 51-8-2

<u>PROJECT</u> :CRC PIT 51-8-2 LAVER JEOMETRY

LAYER # I SURFACE (FT)

X: 0.0 0.0 363.0 390.0 1000.0 Z: 0.0 50.0 50.0 72.0 72.0 X: 0.0 50.0 361.0 72.0 72.0 X: 0.0 50.0 361.0 72.0 1000.0 Z: 0.0 50.1 30.1 305.0 100.0 EAVER 4 3 SUBFACE (FT) X: 0.0 50.2 50.2 305.1 305.1 PHREATIC SUBFACE (FT) X: 0.0 363.0 590.0 1000.0 X: 0.0 363.0 590.0 1000.0 CIACLE CENTER POINTS (FT) HRXENUM SLICE HIDTH (FT): 50.0 RADIUS INCREMENT (FT): 20.0 X: 424.0 Z: 424.0 X: 424.0

April 19, 1985

University of Alberta Department of Civil Engineering Edmonton, Alberta T6G 2G7

Attention: Dr. N.R. Morgenstern, P.Eng.

Dear Dr. Morgenstern:

Subject: M.Eng. Project - Final Report

Enclosed are two final copies of my M.Eng. Project entitled "Case History of an Open Pit Coal Mine Slope Failure at Luscar, Alberta". I understand that the deadline for receipt of these reports by the Faculty of Graduate Studies is April 24, 1985.

I will forward three additional copies for Dr. Cruden, yourself and the Civil Engineering Department within a few days. Thank you very much for your guidance and encouragement in the completion of this project.

Yours truly,

Allmanth

Allan M. MacRae, P.Eng. c/o Canadian Occidental Petroleum Ltd. 1500, 635 8th Avenue S.W. Calgary, Alberta T2P 3Z1 Telephone: 234-6097

AMMcR/slc

Encl.

cc: Mr. Fred Munn, P.Eng. Chief Engineer Cardinal River Coals Ltd. Hinton, Alberta